

Mechanics of Materials



دانشگاه کردستان
University of Kurdistan
زانکۆی کوردستان

Ferdinand P.Beer, E.Russel Johnston, Jr., John T.Dewolf

Other Reference:

J.Wat Oler “Lectures notes on Mechanics od Materials”

Ibrahim A.Assakkaf “Lectures notes on Mechanics od Materials”

Introduction –Concept of Stress

By: Kaveh Karami

Associate Prof. of Structural Engineering

<https://prof.uok.ac.ir/Ka.Karami>

Introduction –Concept of Stress

□ Main objectives:

Mechanics of Materials
answers tow questions:



✓ *Is the material strong enough?*

✓ *Is the material stiff enough?*

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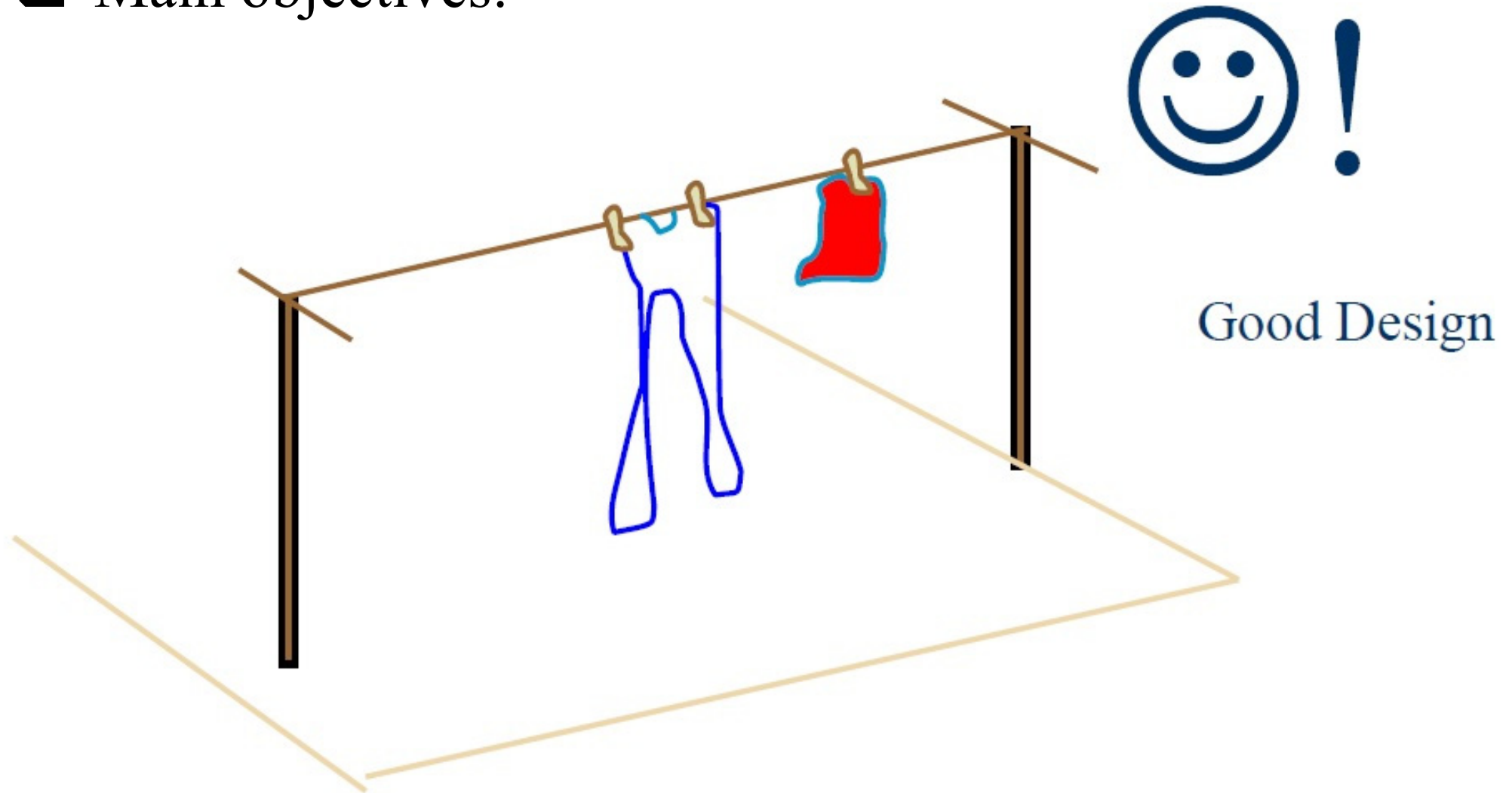
□ Main objectives:

- If the material is not strong enough, your design will break.
- If the material isn't stiff enough, your design probably won't function the way it's intended to.



Introduction –Concept of Stress

□ Main objectives:



Introduction –Concept of Stress

□ Main objectives:



Bad Design



Introduction –Concept of Stress

□ Main objectives:

- ❖ Provide the future engineer with the **means of analyzing and designing** various machines and load bearing structures.
- ❖ Both the analysis and design of a given structure involve the **determination of *stresses* and *deformations***. This chapter is devoted to the concept of stress.

Introduction –Concept of Stress

- Design Considerations:
 - Safety.
 - Economy.



Design of Engineering systems is usually trade off between maximizing safety and minimizing cost.

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- Typical approach to Engineering solutions:
 - ✓ Identify the problem.
 - ✓ State the objective.
 - ✓ Develop alternative solutions.
 - ✓ Evaluate alternatives.
 - ✓ Use the best alternative.



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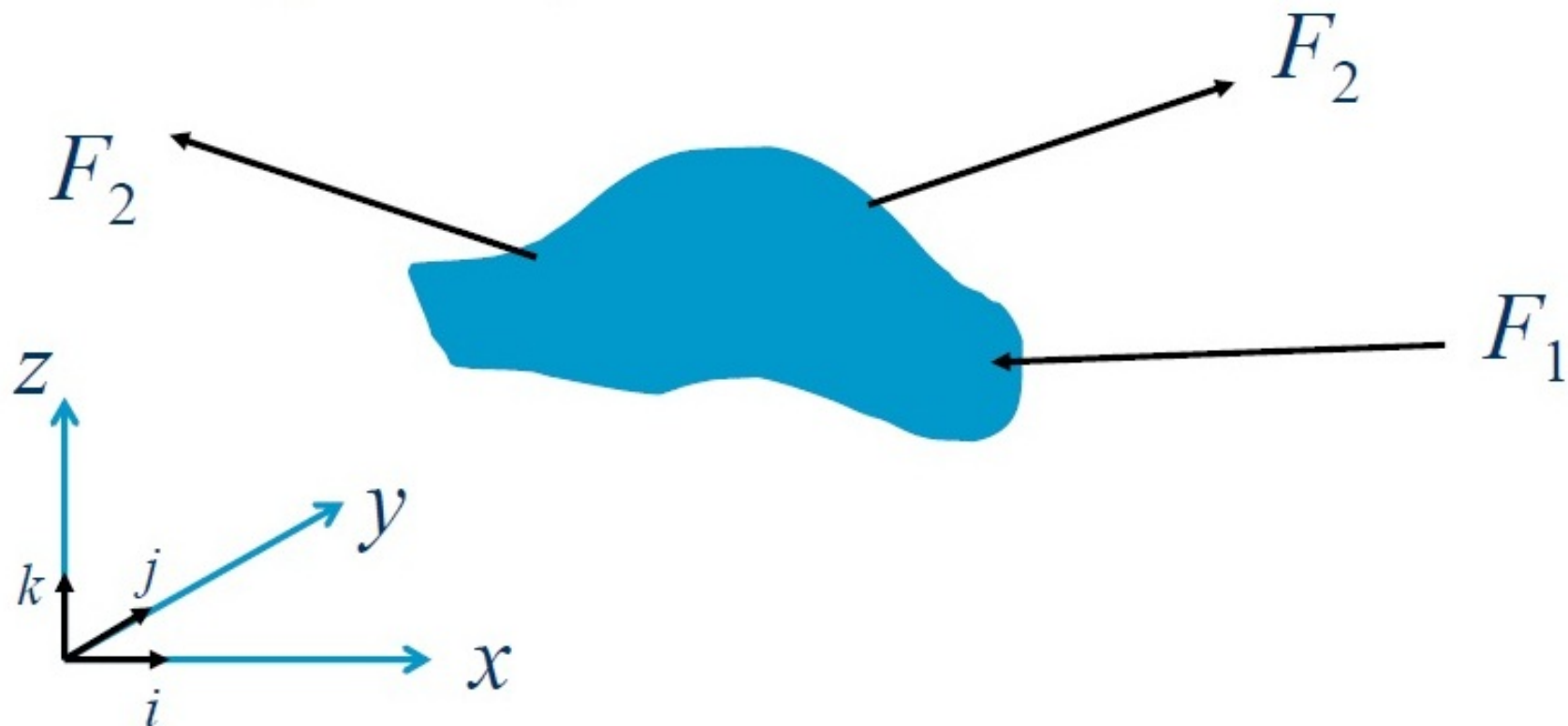
□ Steps of Analysis

- ✓ Equations of equilibrium are used for external forces.
- ✓ Analysis of the effect of the external forces on the structures (machine) or any component of the structure (machine).
- ✓ Behavior of the materials under the action of forces.

Introduction – Concept of Stress

□ Equations of Equilibrium

- Rigid Body



Introduction –Concept of Stress

□ Equations of Equilibrium

- For a rigid body to be in equilibrium, both the resultant force **R** and a resultant moments (couples) **C** must vanish.

$$\mathbf{R} = \sum F_x \mathbf{i} + \sum F_y \mathbf{j} + \sum F_z \mathbf{k} = \mathbf{0}$$

$$\mathbf{C} = \sum M_x \mathbf{i} + \sum M_y \mathbf{j} + \sum M_z \mathbf{k} = \mathbf{0}$$

Introduction – Concept of Stress

□ Equations of Equilibrium

- The two conditions can also be expressed in scalar form as:

$$\begin{array}{ccc} \sum F_x = 0 & \sum F_y = 0 & \sum F_z = 0 \\ \sum M_x = 0 & \sum M_y = 0 & \sum M_z = 0 \end{array}$$

Introduction –Concept of Stress

□ Equilibrium in Two Dimensions

- The term “two dimensional” is used to describe problems in which the forces under consideration are contained in a plane (say the xy -plane)



Introduction –Concept of Stress

□ Equilibrium in Two Dimensions

- For two-dimensional problems, since a force in the xy-plane has no z-component and produces no moments about the x or y axes, hence

$$\mathbf{R} = \sum F_x \mathbf{i} + \sum F_y \mathbf{j} = \mathbf{0}$$

$$\mathbf{C} = \sum M_z \mathbf{k} = \mathbf{0}$$

Introduction –Concept of Stress

□ Equilibrium in Two Dimensions

- In scalar form, these conditions can be expressed as

$$\begin{array}{ccc} \sum F_x = 0 & & \sum F_y = 0 \\ & \sum M_z = 0 & \end{array}$$

Introduction –Concept of Stress

□ Internal Forces for Axially Loaded Members

- Analysis of Internal Forces

Example 1



Assume that: $F_1 = 2 \text{ k}$, $F_3 = 5 \text{ k}$, and $F_4 = 8 \text{ k}$
 $F_2 = ?$

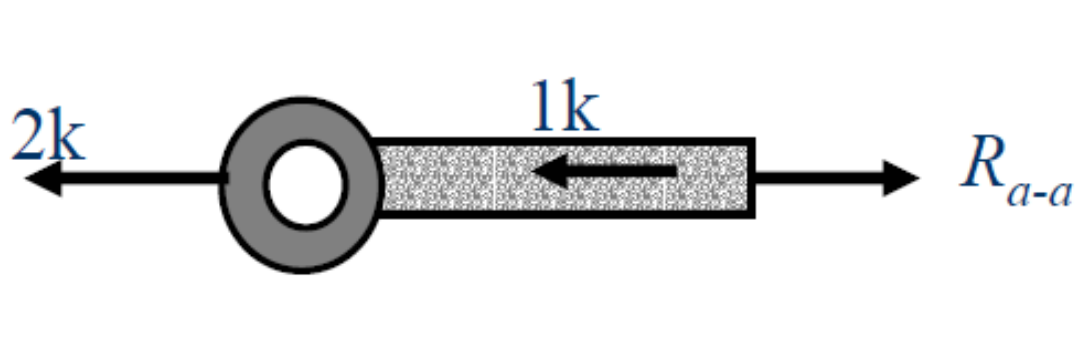
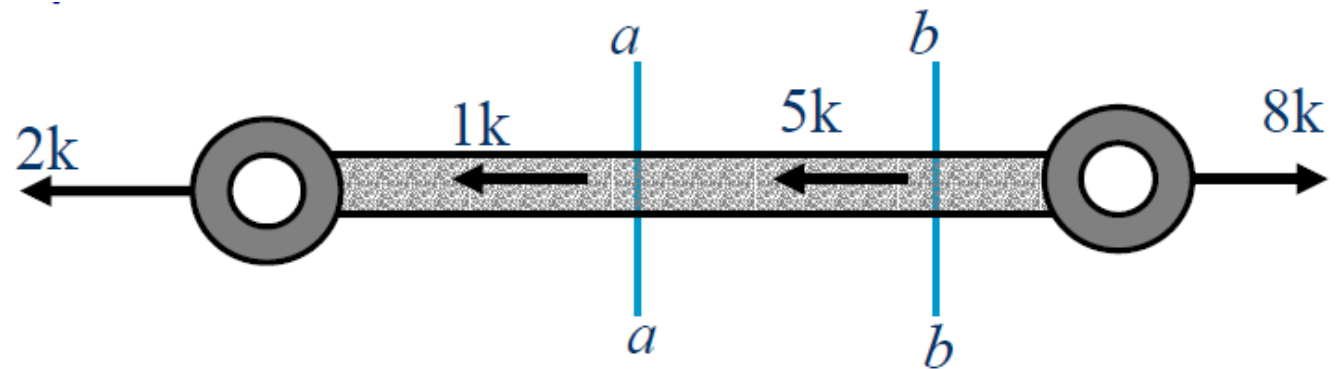
Introduction – Concept of Stress

□ Internal Forces for Axially Loaded Members

- Analysis of Internal Forces

- What is the internal force developed on plane a-a and b-b?

Example 2

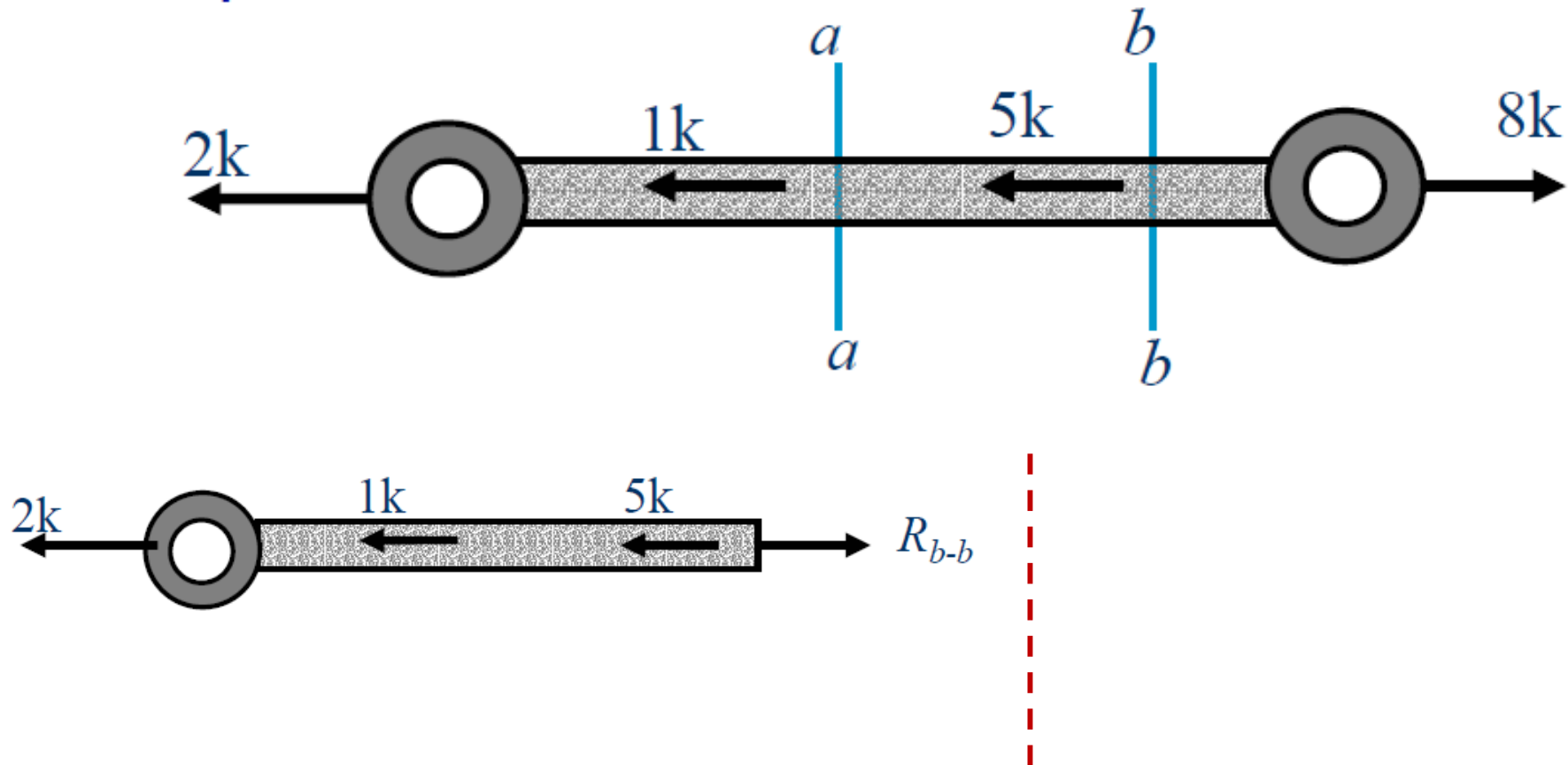


Introduction – Concept of Stress

□ Internal Forces for Axially Loaded Members

- Analysis of Internal Forces

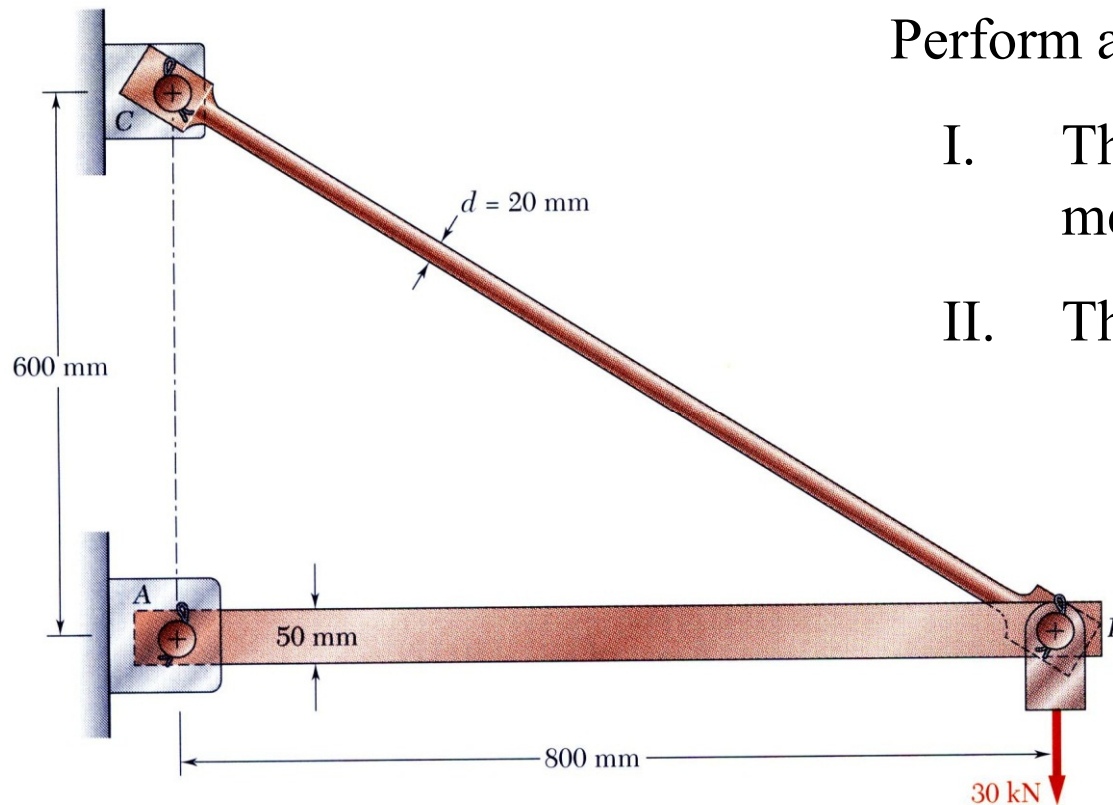
Example 2



Introduction – Concept of Stress

□ Internal Forces for Axially Loaded Members

Example 3



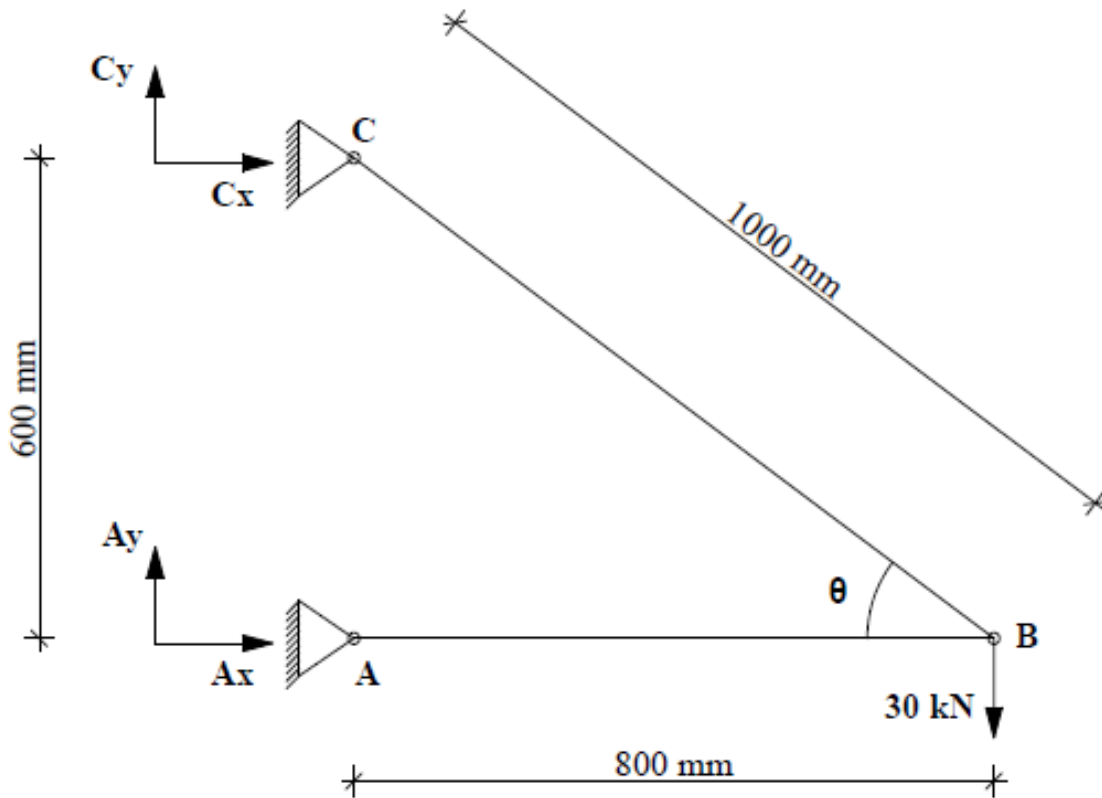
Perform a static analysis to determine:

- I. The internal force in each structural member
- II. The reaction forces at the supports.

Introduction – Concept of Stress

□ Internal Forces for Axially Loaded Members

Example 3



Three empty rounded rectangular boxes for calculations or answers.

Introduction – Concept of Stress

□ Internal Forces for Axially Loaded Members

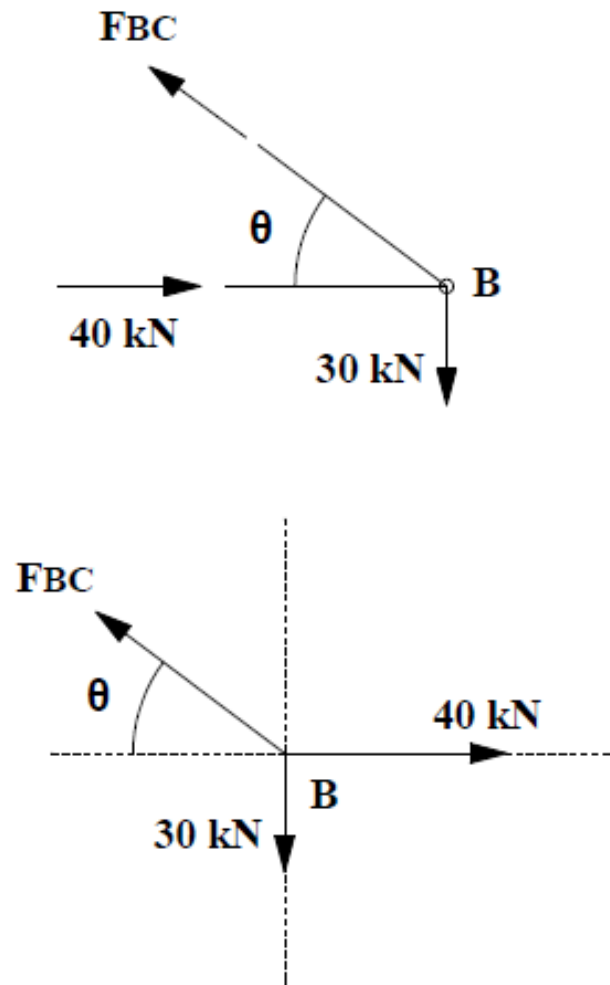
Example 3



Introduction – Concept of Stress

□ Internal Forces for Axially Loaded Members

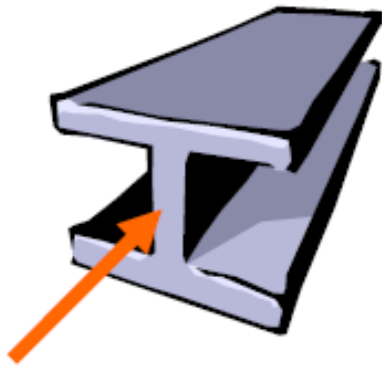
Example 3



Introduction –Concept of Stress

□ Axial Loading: Normal Stress

- Stress
 - Stress is the intensity of internal force.
 - It can also be defined as force per unit area, or intensity of the forces distributed over a given section.

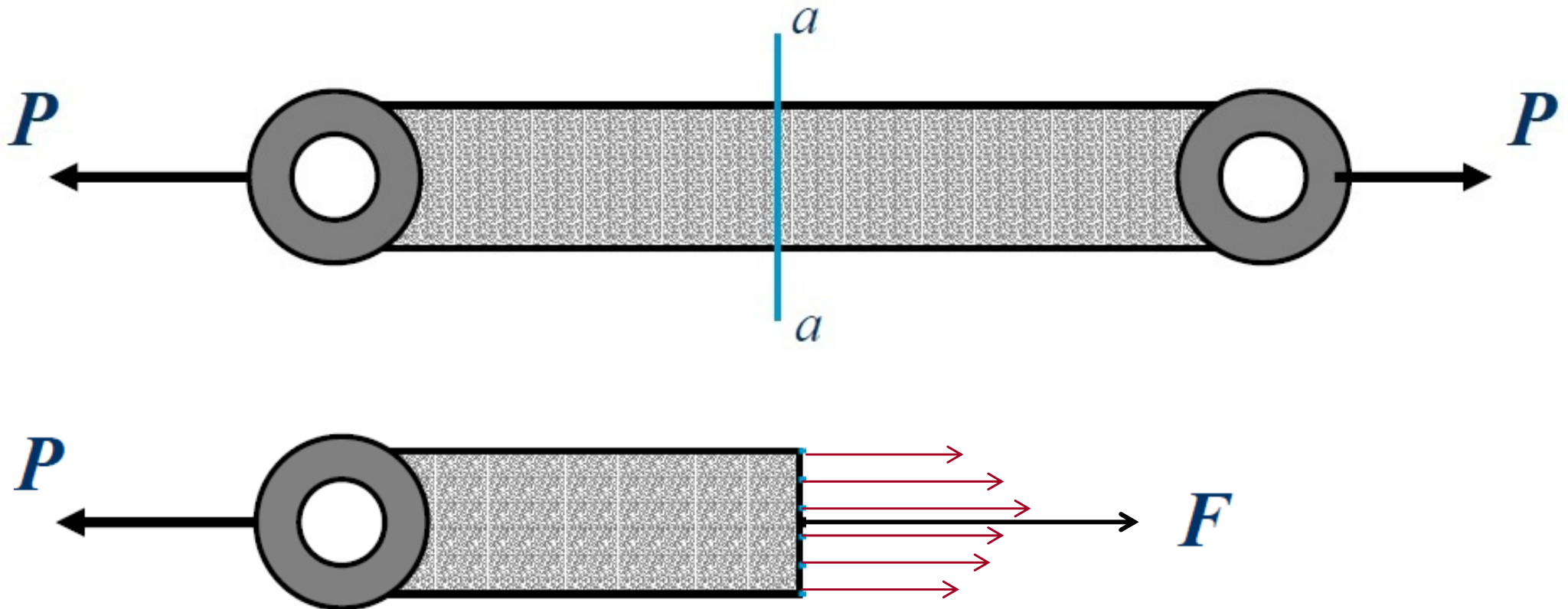


$$\text{Stress} = \frac{\text{Force}}{\text{Area}}$$

Introduction – Concept of Stress

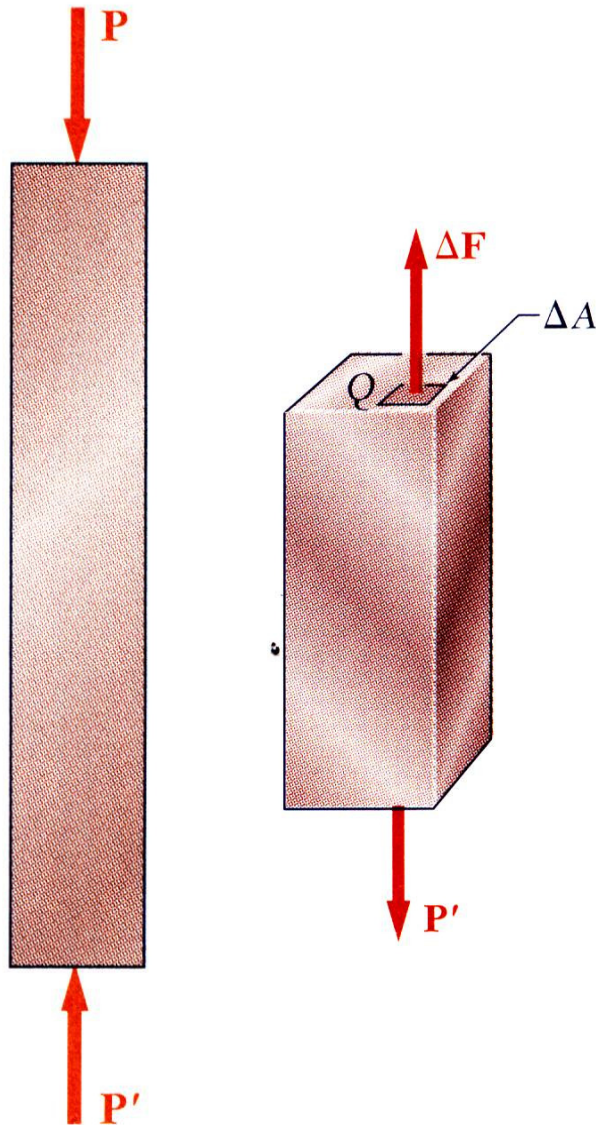
□ Axial Loading: Normal Stress

- The resultant of the internal forces for an axially loaded member is *normal* to a section cut perpendicular to the member axis.



Introduction – Concept of Stress

□ Axial Loading: Normal Stress

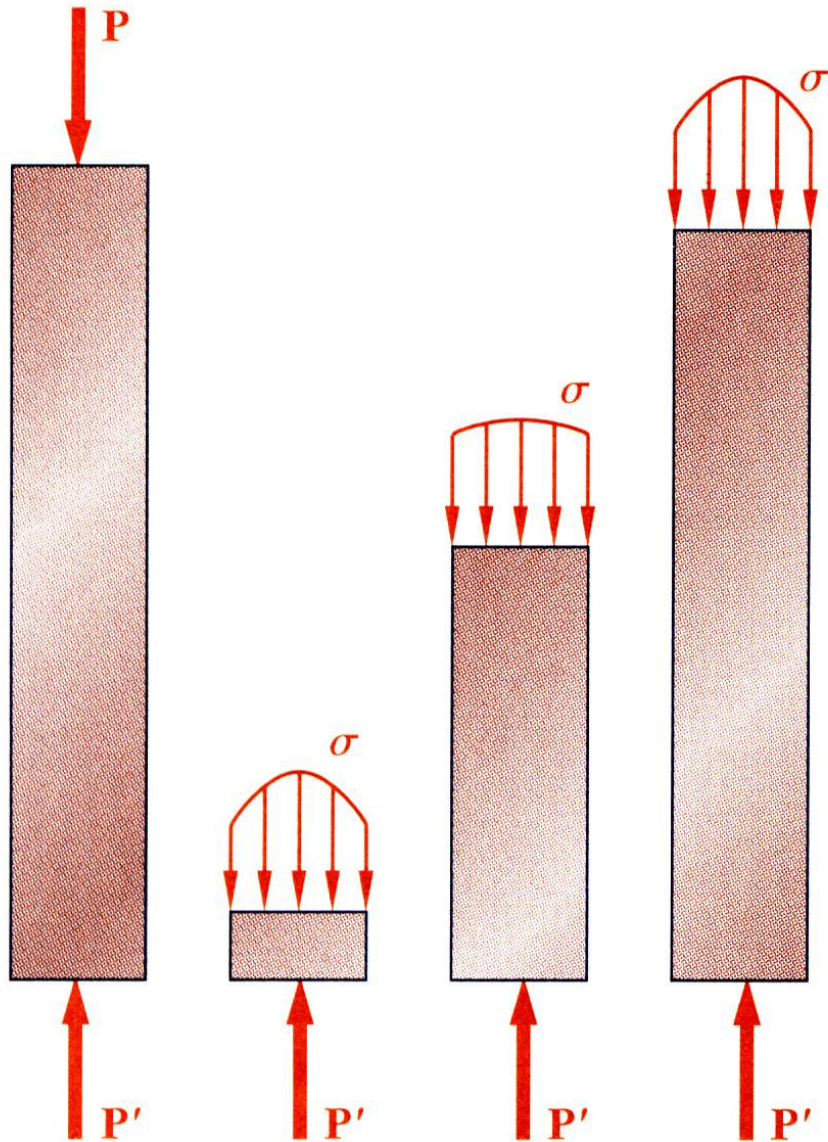


- The force intensity on that section is defined as the normal stress.

$$\sigma = \lim_{\Delta A \rightarrow 0} \frac{\Delta F}{\Delta A} \quad \sigma_{ave} = \frac{P}{A}$$

Introduction – Concept of Stress

□ Axial Loading: Normal Stress



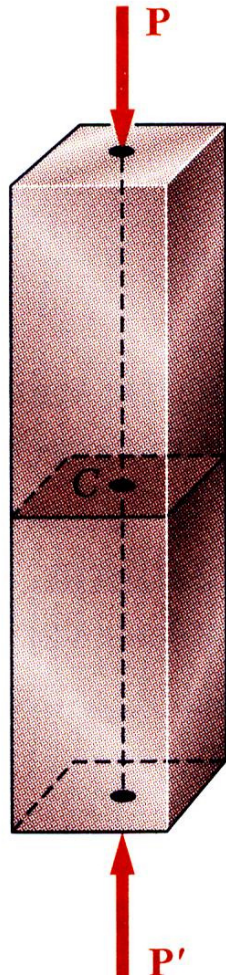
- The normal stress at a particular point may not be equal to the average stress but the resultant of the stress distribution must satisfy

$$\sigma = \lim_{\Delta A \rightarrow 0} \frac{\Delta F}{\Delta A} \Rightarrow \sigma = \frac{dF}{dA}$$

$$P = \int dF = \int_A \sigma dA$$

Introduction –Concept of Stress

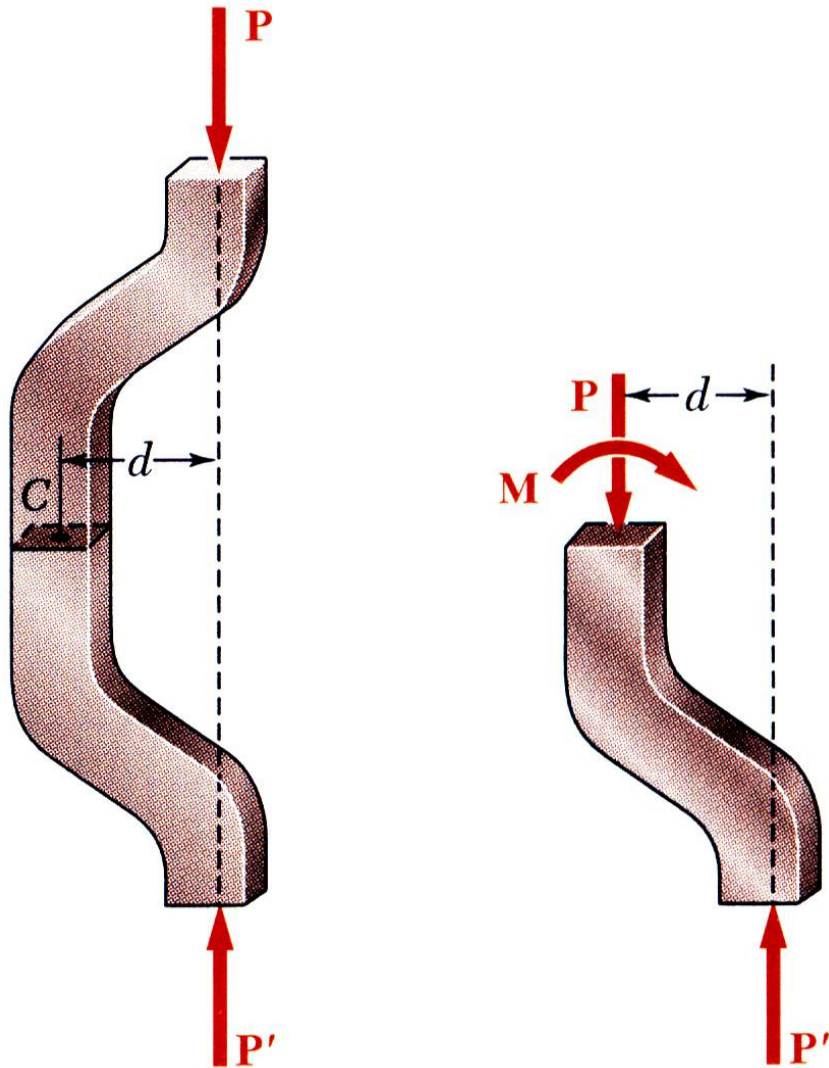
□ Centric loading



- A uniform distribution of stress is only possible if the concentrated loads on the end sections of two-force members are applied at the section centroids. This is referred to as *centric loading*.

Introduction – Concept of Stress

□ Eccentrically loaded

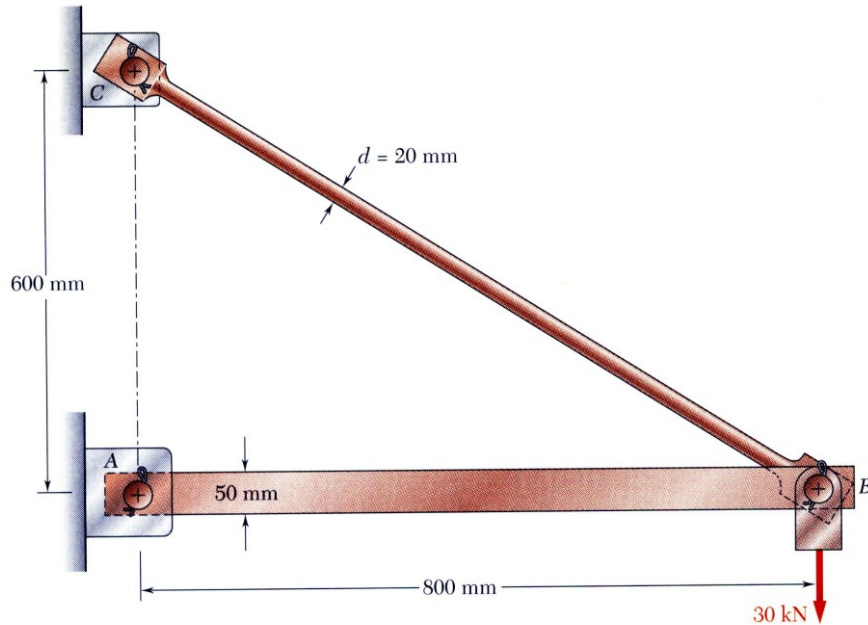


- If a two-force member is ***eccentrically loaded***, then the resultant of the stress distribution in a section must yield an axial force and a moment.

Introduction – Concept of Stress

□ Stress Analysis & Design Example

Example 4

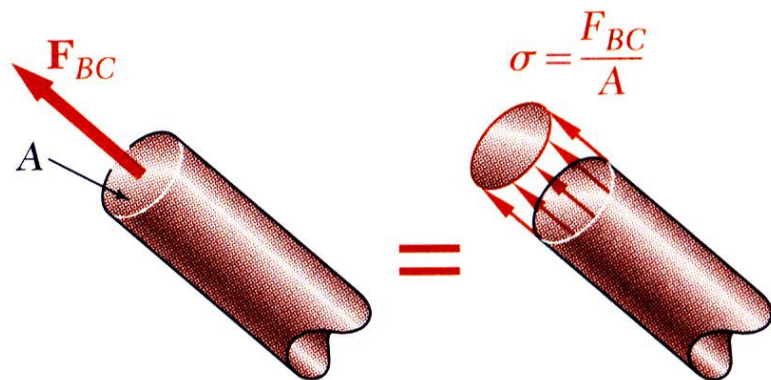


Can the structure safely support the 30 kN load? Assume that, the element CB has enough capacity. $\sigma_{\text{all } AB} = 165$ MPa

- From a statics analysis

$$F_{AB} = 40 \text{ kN (compression)}$$

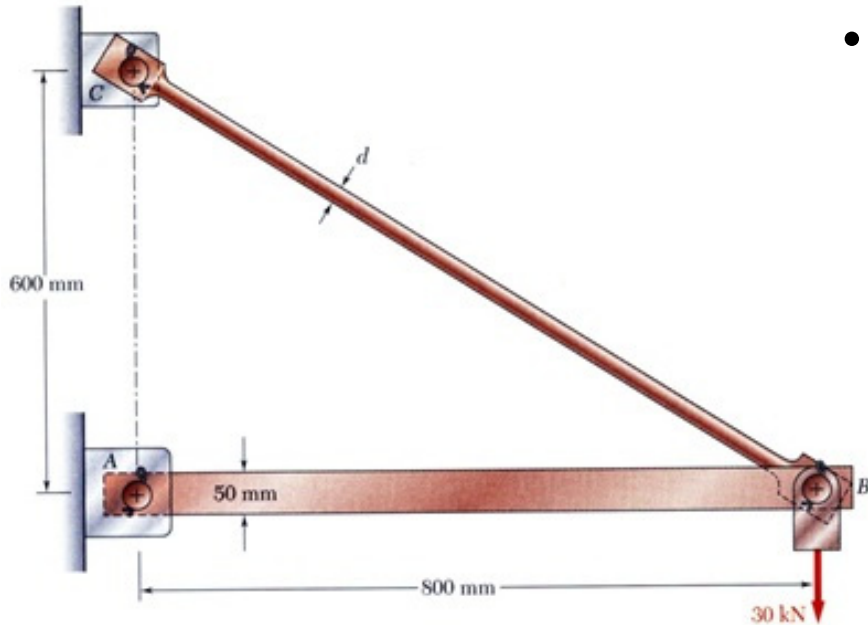
$$F_{BC} = 50 \text{ kN (tension)}$$



Introduction –Concept of Stress

□ Stress Analysis & Design Example

Example 4



- For reasons based on cost, weight, availability, etc., the choice is made to construct the rod from aluminum ($\sigma_{all_{AB}} = 100 \text{ MPa}$). What is an appropriate choice for the rod diameter?

- From a statics analysis

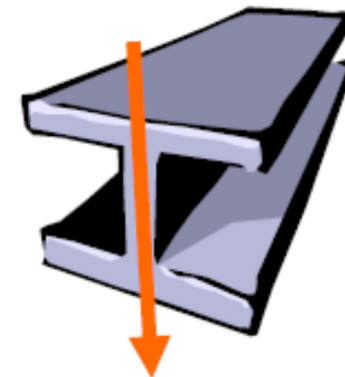
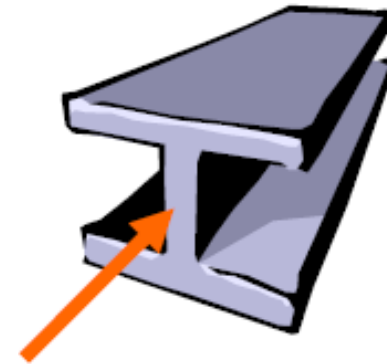
$$F_{AB} = 40 \text{ kN (compression)}$$

$$F_{BC} = 50 \text{ kN (tension)}$$

Introduction –Concept of Stress

□ Shearing Stress

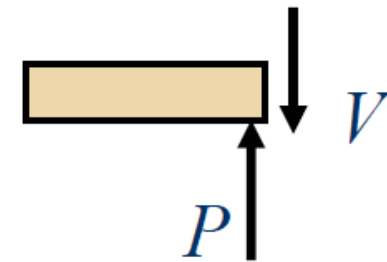
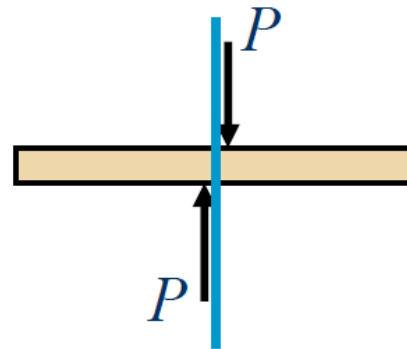
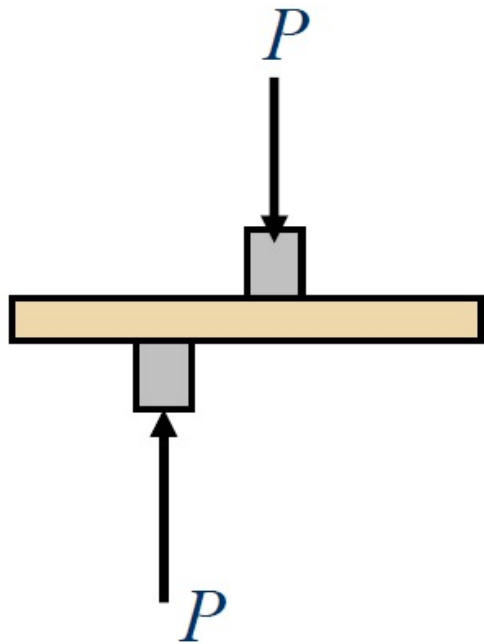
- The internal forces discussed previously and the corresponding stresses were *normal* to the section considered.
- A different type of stress can occur in a **transverse** cross section of a member as shown in the next slide.



Introduction – Concept of Stress

□ Shearing Stress

- Illustration of Shearing Stress



$$\tau_{\text{ave}} = \frac{V}{A_s} = \frac{P}{A_s}$$

Introduction –Concept of Stress

□ Shearing Stress

- Shear

$$\tau = \lim_{\Delta A_s \rightarrow 0} \frac{\Delta V}{\Delta A_s} \Rightarrow \tau = \frac{dV}{dA_s}$$

$$P = \int dV = \int_{A_s} \tau dA_s$$

A_s = cross-sectional area of bolt or rivet

- Shear stress distribution varies from zero at the member surfaces to maximum values that may be much larger than the average value.
- The shear stress distribution cannot be assumed to be uniform.

Introduction –Concept of Stress

□ Shearing Stress

Shearing Stress in Connection

Shearing stresses are commonly found in bolts, pins, and rivets used to connect various structural members and machine components.

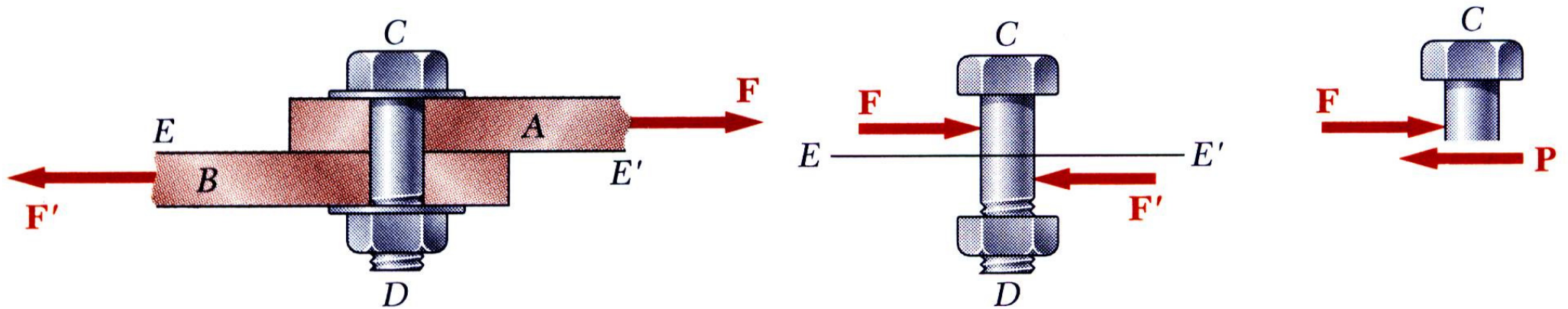
Three Types of Shearing Stress

- a) Single Shear
- b) Double Shear
- c) Punching Shear

Introduction – Concept of Stress

□ Shearing Stress Examples

▪ Single Shear



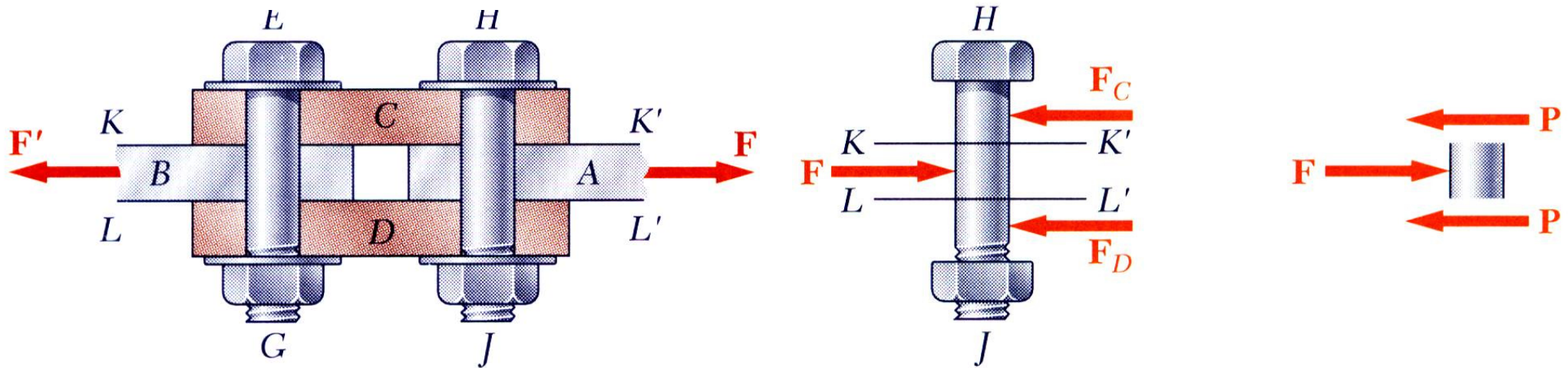
The bolt is said to in single shear P

$$\tau_{\text{ave}} = \frac{P}{A_s} = \frac{F}{A_s}$$

Introduction – Concept of Stress

□ Shearing Stress Examples

▪ Double Shear



The bolt is said to be in double shear P

$$\tau_{\text{ave}} = \frac{P}{A_s} = \frac{F/2}{A_s} = \frac{F}{2A_s}$$

Introduction –Concept of Stress

□ Shearing Stress Examples

▪ **Punching Shear**

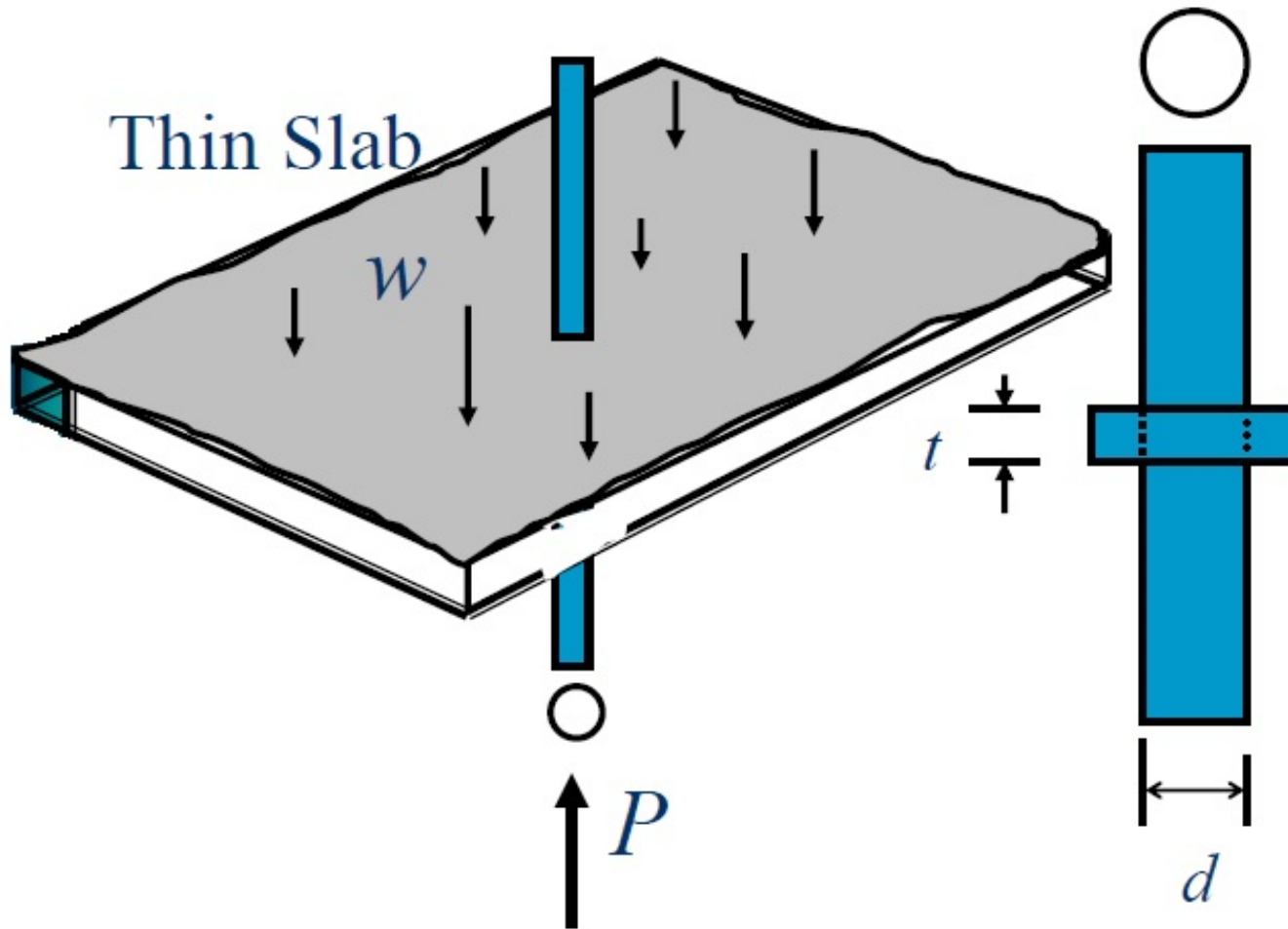
Examples of this type are:

- Action of punch in forming rivet hole in metal
- Tendency of building columns to punch through footings
- Heavy thin-slab ceiling cause building columns to punch through slabs.

Introduction – Concept of Stress

□ Shearing Stress Examples

▪ Punching Shear



$$A_s = \pi dt$$

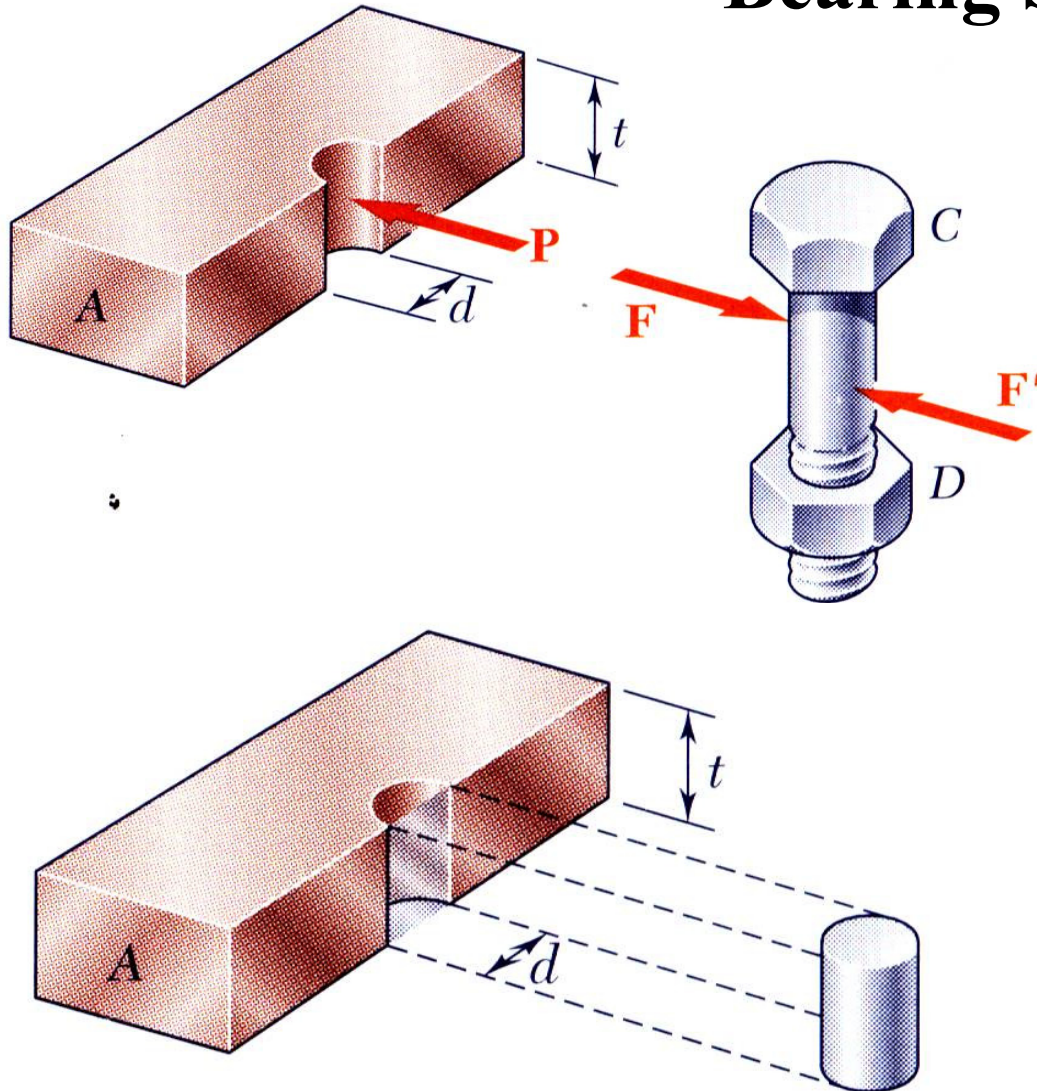


$$\tau_{\text{avg}} = \frac{P}{A_s} = \frac{P}{\pi dt}$$

Introduction –Concept of Stress

□ Shearing Stress Examples

▪ Bearing Stress in Connections



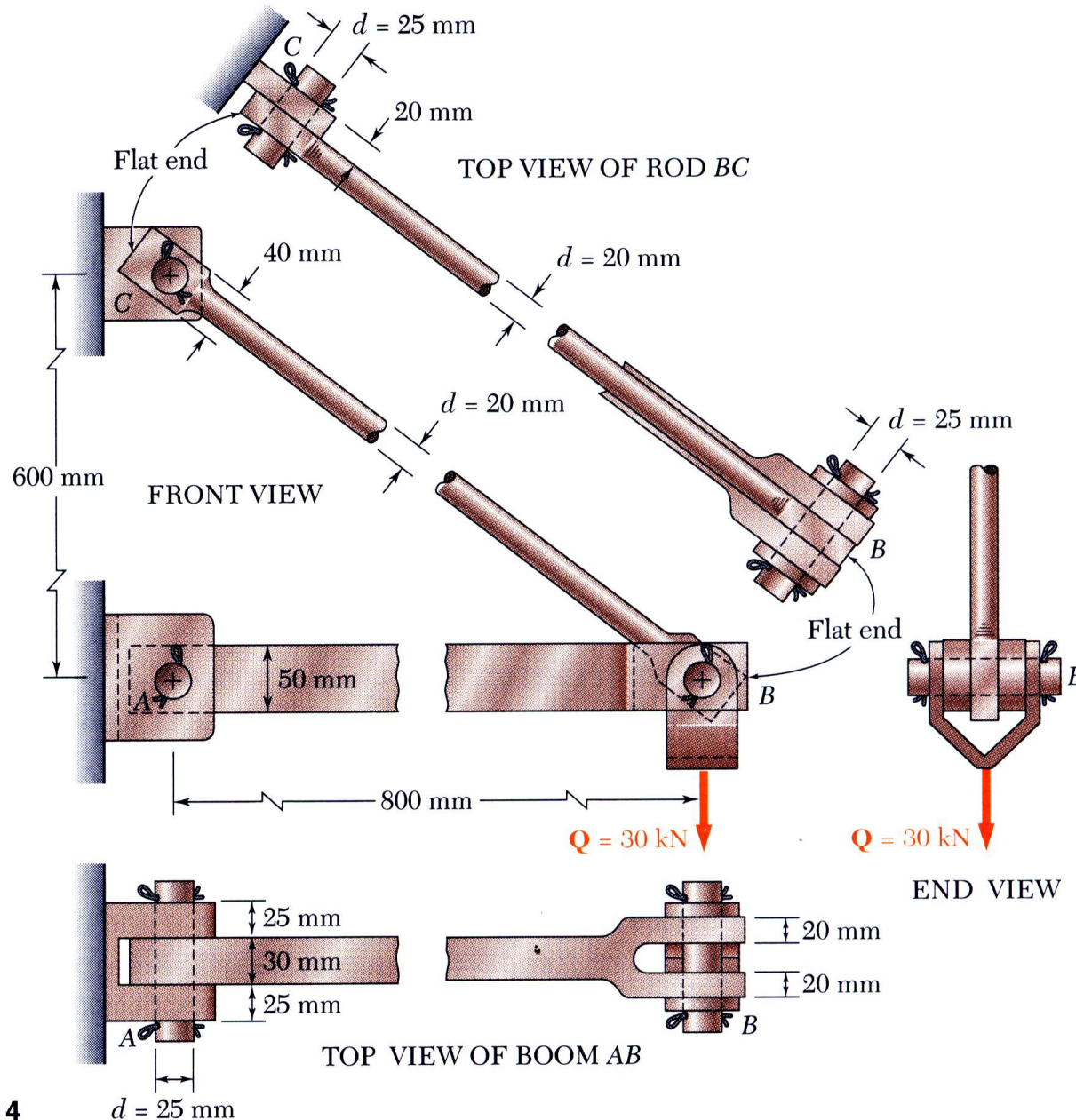
- Bolts, rivets, and pins create stresses on the points of contact or *bearing surfaces* of the members they connect.
- Corresponding average force intensity is called the **bearing stress**,

$$\sigma_b = \frac{P}{A} = \frac{P}{t d}$$

Introduction – Concept of Stress

Example 5

□ Stress Analysis & Design Example



- Determine the stresses in the members and connections of the structure shown.

- From a statics analysis

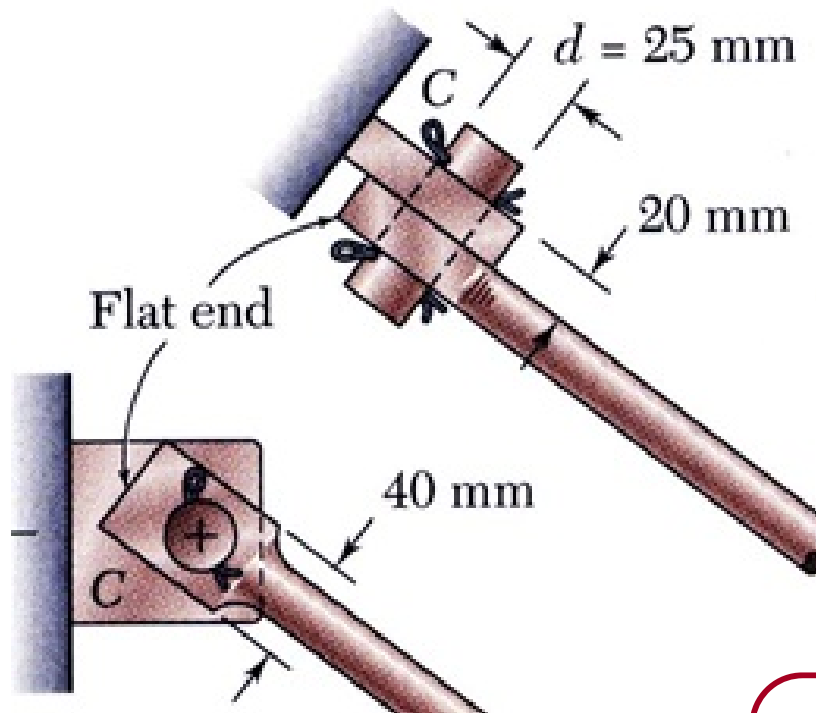
$$F_{AB} = 40 \text{ kN (compression)}$$

$$F_{BC} = 50 \text{ kN (tension)}$$

Introduction – Concept of Stress

□ Rod & Boom Normal Stresses

Example 5



$$F_{BC} = 50 \text{ kN (tension)}$$

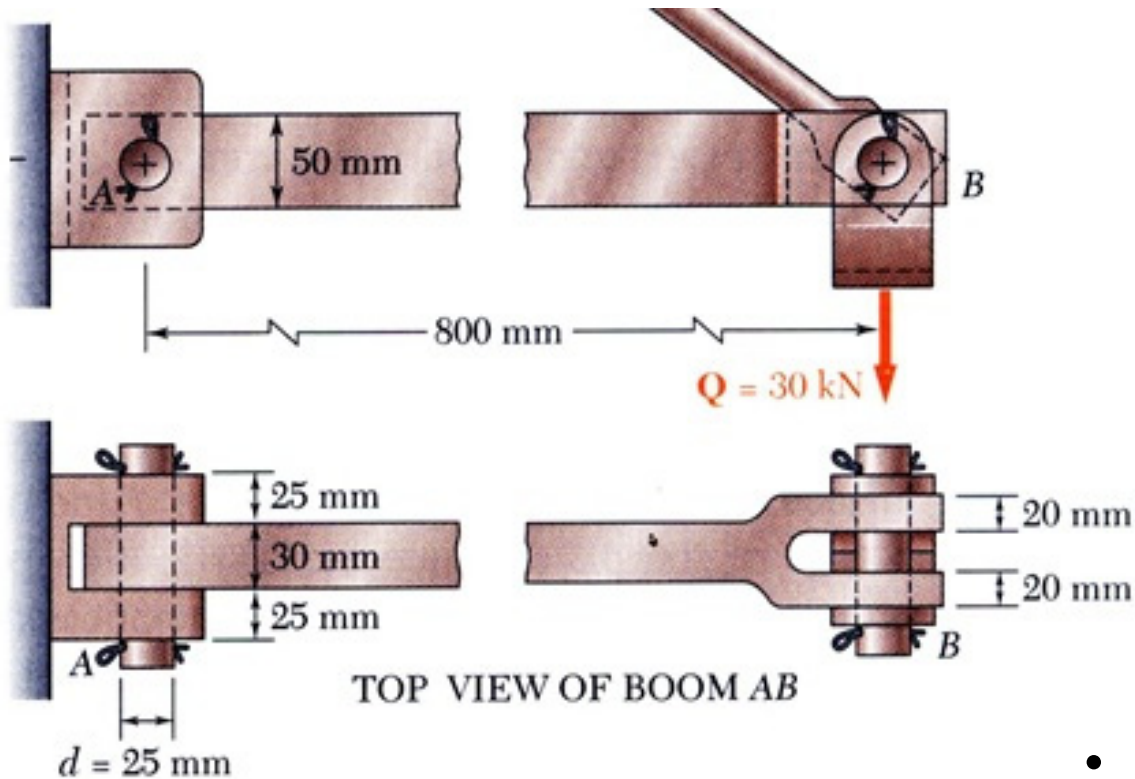
- At the rod center, the average normal stress in the circular cross-section.

- At the flattened rod ends, the smallest cross-sectional area occurs at the pin centerline.

Introduction – Concept of Stress

□ Rod & Boom Normal Stresses

Example 5



- The average normal stress in the center boom.



$$F_{AB} = 40 \text{ kN (compression)}$$

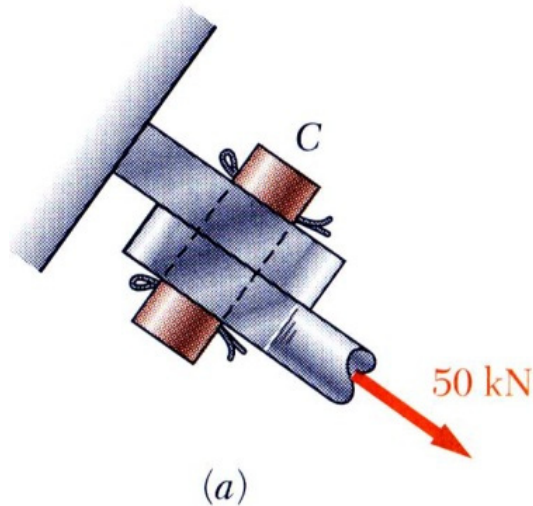
- The minimum area sections at the boom ends are unstressed since the boom is in compression.

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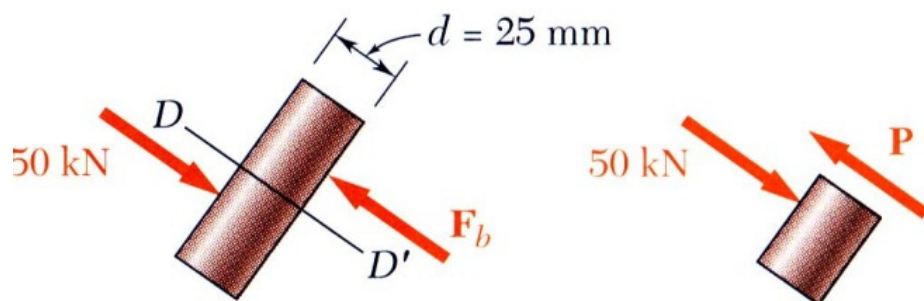
Example 5

□ Pin Shearing Stresses

- The cross-sectional area for pins at A , B , and C ,



- The force on the pin at C is equal to the force exerted by the rod BC ,

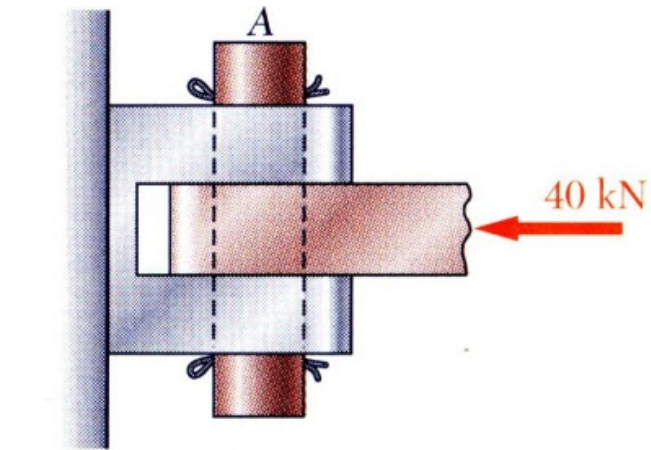


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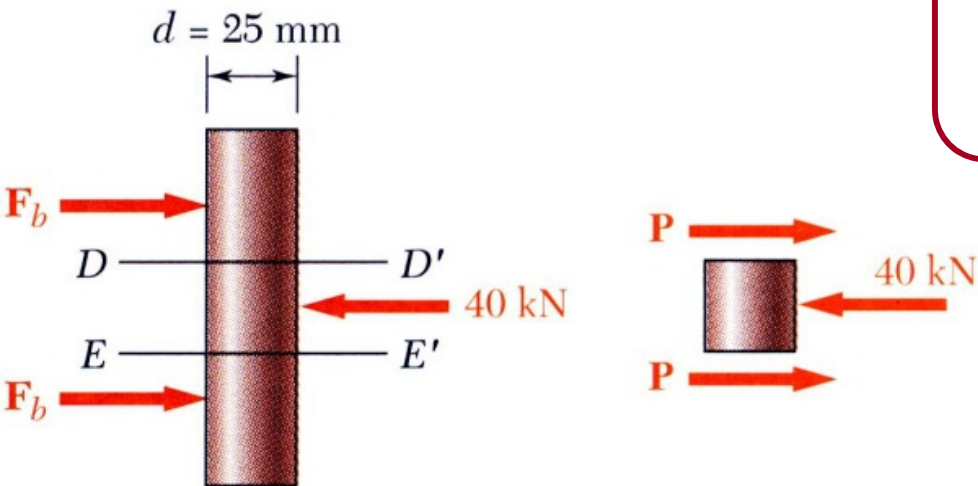
Example 5

□ Pin Shearing Stresses

- The pin at A is in double shear with a total force equal to the force exerted by the boom AB ,



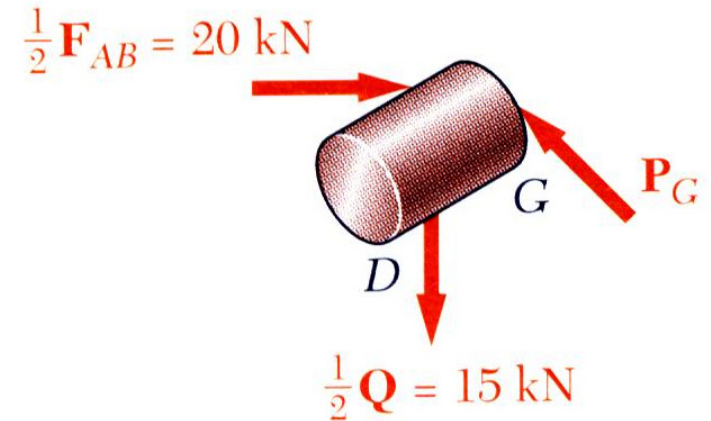
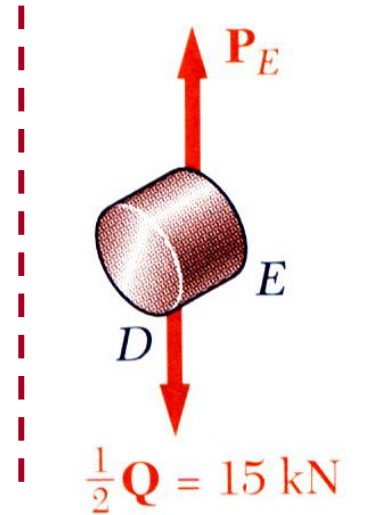
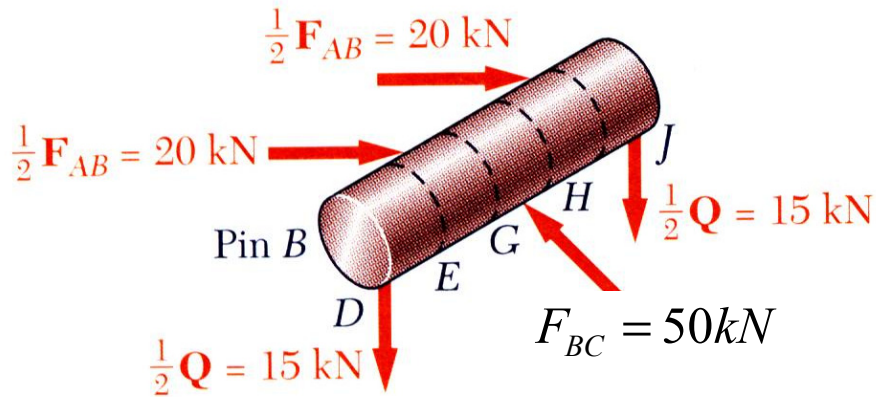
(a)



Introduction – Concept of Stress

Example 5

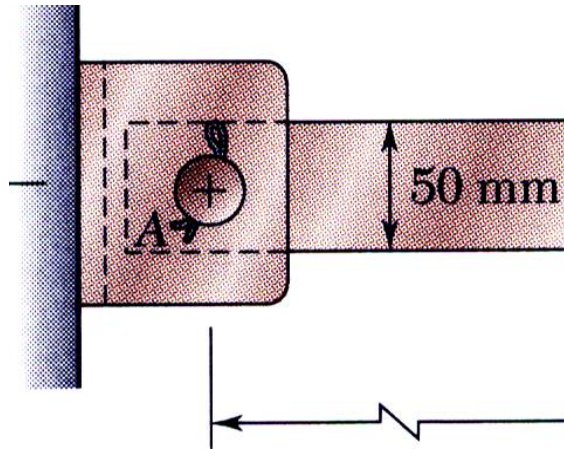
□ Pin Shearing Stresses



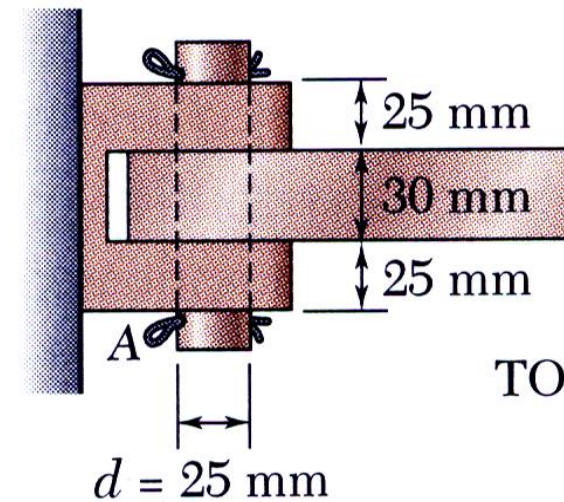
Introduction – Concept of Stress

Example 5

□ Pin Shearing Stresses



- To determine the bearing stress at A in the boom AB , we have $t = 30$ mm and $d = 25$ mm,



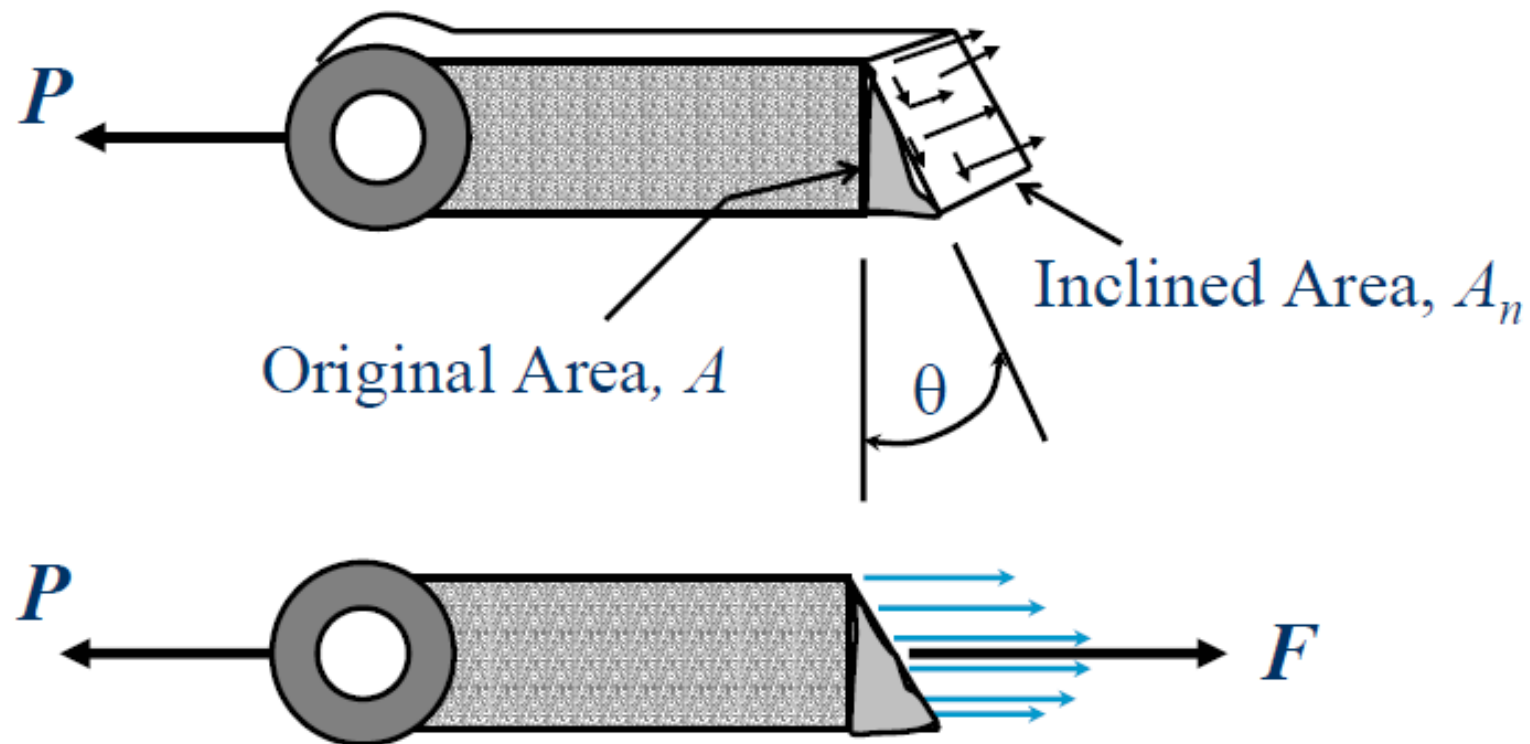
- To determine the bearing stress at A in the bracket, we have $t = 2(25 \text{ mm}) = 50$ mm and $d = 25$ mm,



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□ Stresses on an Inclined Plane in an Axially Loaded Member

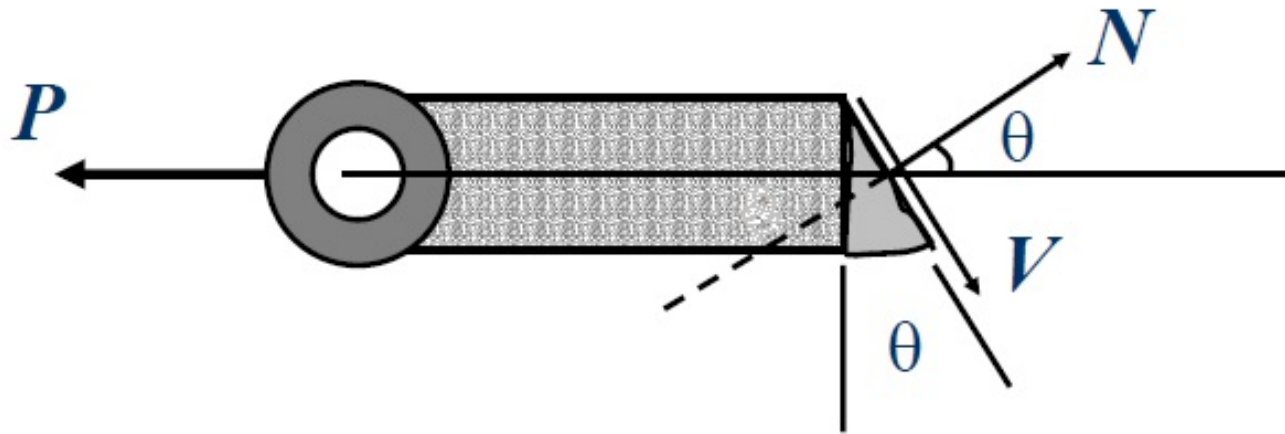
▪ Illustration



Introduction –Concept of Stress

□ Stresses on an Inclined Plane in an Axially Loaded Member

■ Illustration



$$V = P \sin(\theta)$$

$$N = P \cos(\theta)$$

$$A_n = \frac{A}{\cos(\theta)}$$

$$\sigma_n = \frac{N}{A_n} = \frac{P \cos(\theta)}{A / \cos(\theta)} = \frac{P}{A} \cos^2(\theta) = \frac{P}{2A} (1 + \cos(2\theta))$$

$$\tau_n = \frac{V}{A_n} = \frac{P \sin(\theta)}{A / \cos(\theta)} = \frac{P}{A} \sin(\theta) \cos(\theta) = \frac{P}{2A} \sin(2\theta)$$

Introduction –Concept of Stress

- Stresses on an Inclined Plane in an Axially Loaded Member
 - Maximum Normal and Shear Stresses

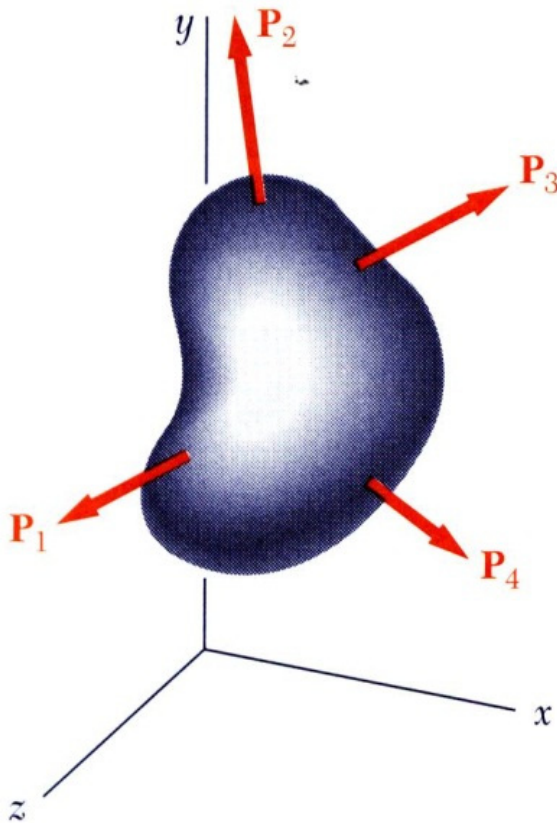
$$\theta = 0^\circ \text{ or } 180^\circ \Rightarrow \sigma_n \text{ is maximum} \Rightarrow \sigma_{nMax} = \frac{P}{A}$$

$$\theta = 45^\circ \text{ or } 135^\circ \Rightarrow \tau_n \text{ is maximum} \Rightarrow \tau_{nMax} = \frac{P}{2A}$$

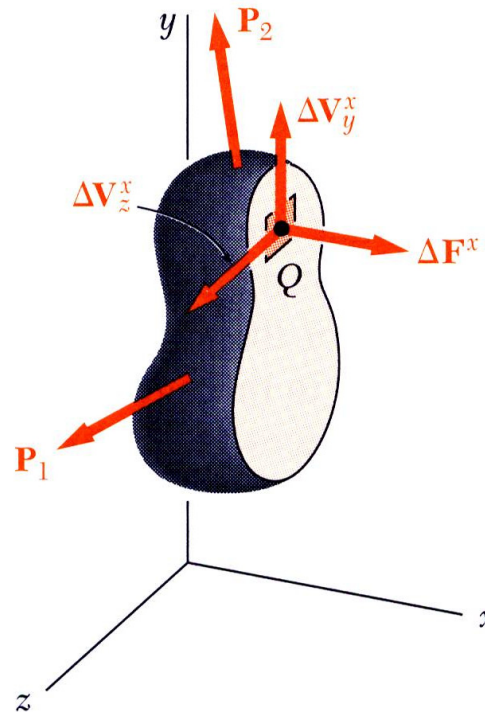
$$\Rightarrow \tau_{nMax} = \frac{\sigma_{nMax}}{2}$$

Introduction – Concept of Stress

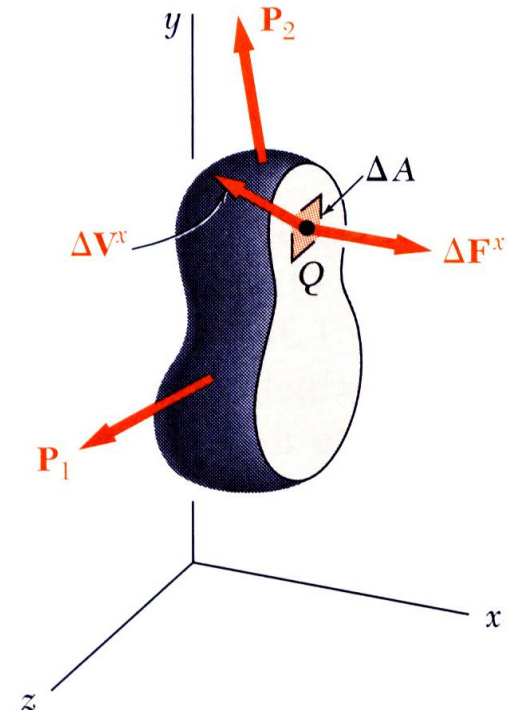
□ Stress Under General Loadings



$$\sigma_x = \lim_{\Delta A \rightarrow 0} \frac{\Delta F^x}{\Delta A}$$



$$\tau_{xy} = \lim_{\Delta A \rightarrow 0} \frac{\Delta V_y^x}{\Delta A}$$

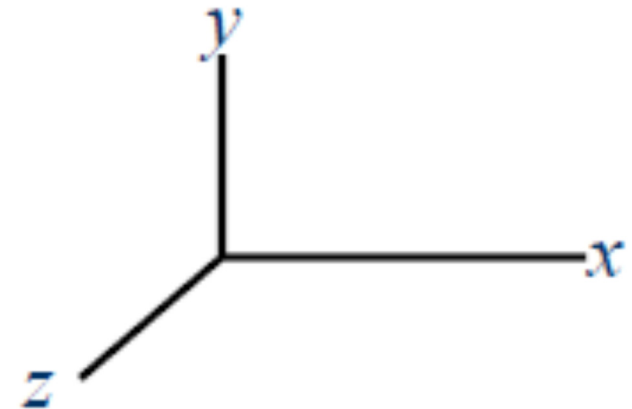
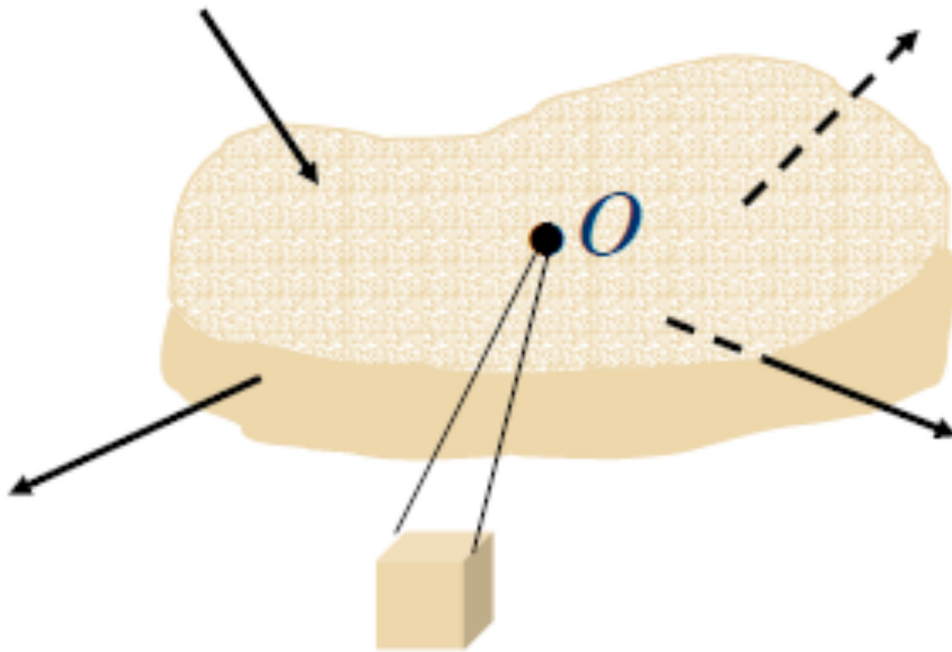


$$\tau_{xz} = \lim_{\Delta A \rightarrow 0} \frac{\Delta V_z^x}{\Delta A}$$

Transformations of Stress and Strain

□ State of Stress

Stress at a point in a material body has been defined as a force per unit area. But this definition is somewhat ambiguous since it depends upon *what area we consider at the point*.

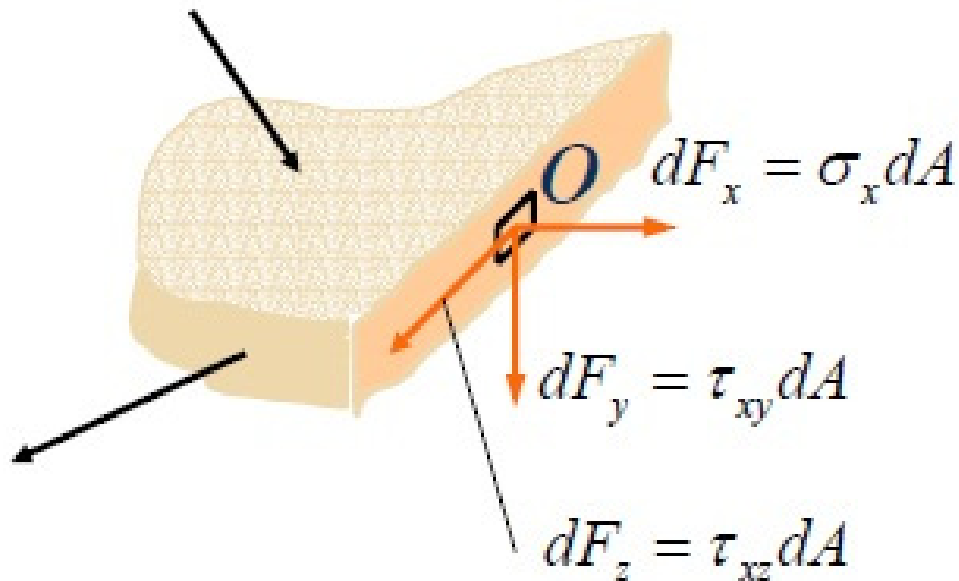


Transformations of Stress and Strain

□ State of Stress

let us pass a cutting plane through point O perpendicular to the x axis.

If dA is the area, then by definition



$$\sigma_x = \frac{dF_x}{dA}$$

$$\tau_{xy} = \frac{dF_y}{dA}$$

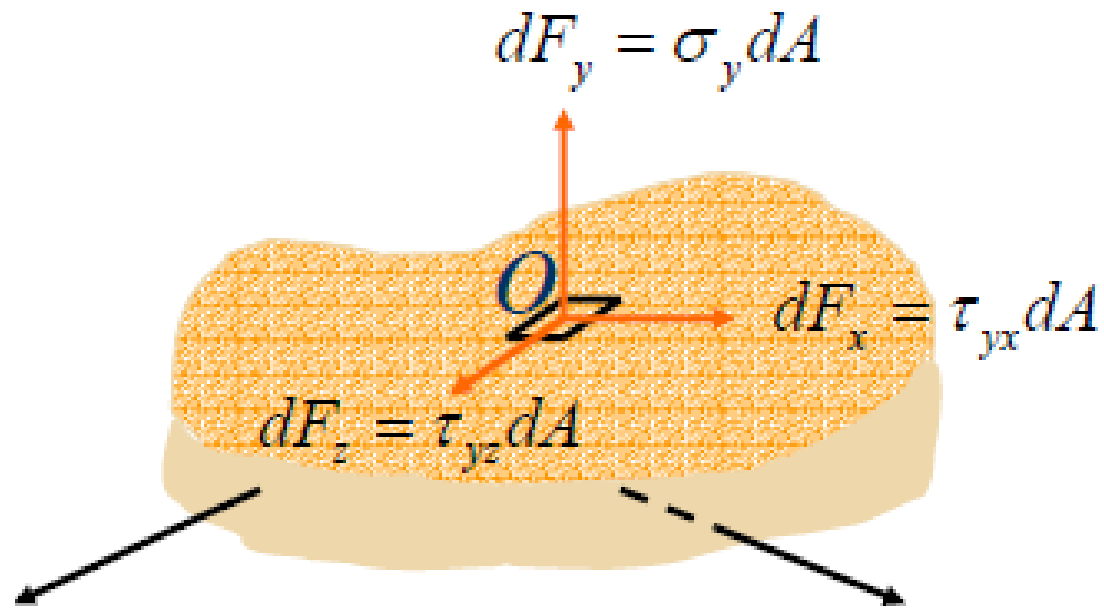
$$\tau_{xz} = \frac{dF_z}{dA}$$

Transformations of Stress and Strain

□ State of Stress

let us pass a cutting plane through point O perpendicular to the y axis.

If dA is the area, then by definition



$$\sigma_y = \frac{dF_y}{dA}$$

$$\tau_{yx} = \frac{dF_x}{dA}$$

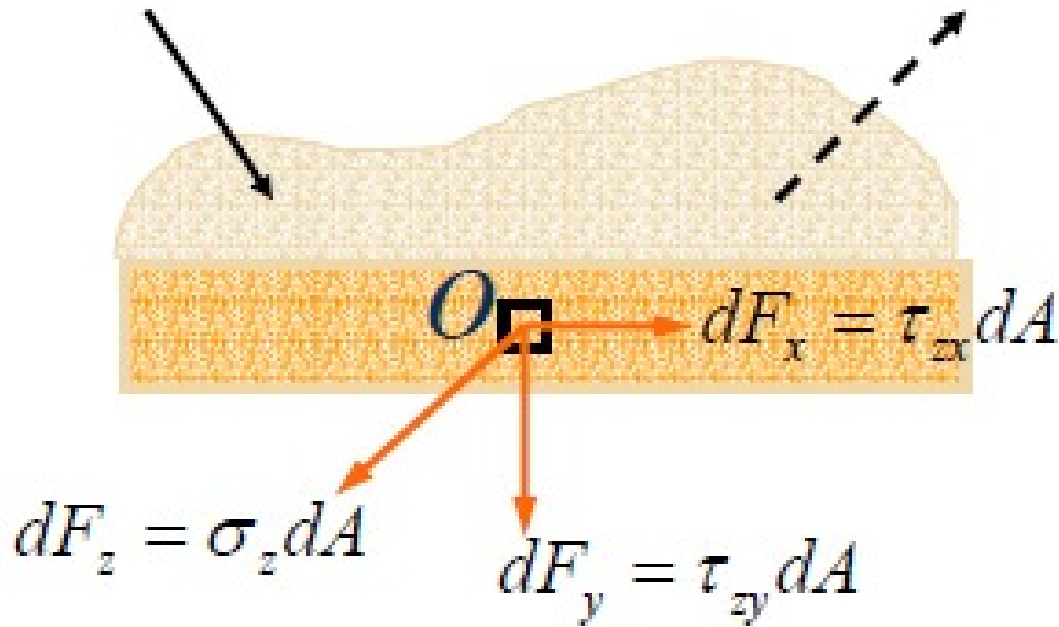
$$\tau_{yz} = \frac{dF_z}{dA}$$

Transformations of Stress and Strain

□ State of Stress

let us pass a cutting plane through point O perpendicular to the z axis.

If dA is the area, then by definition



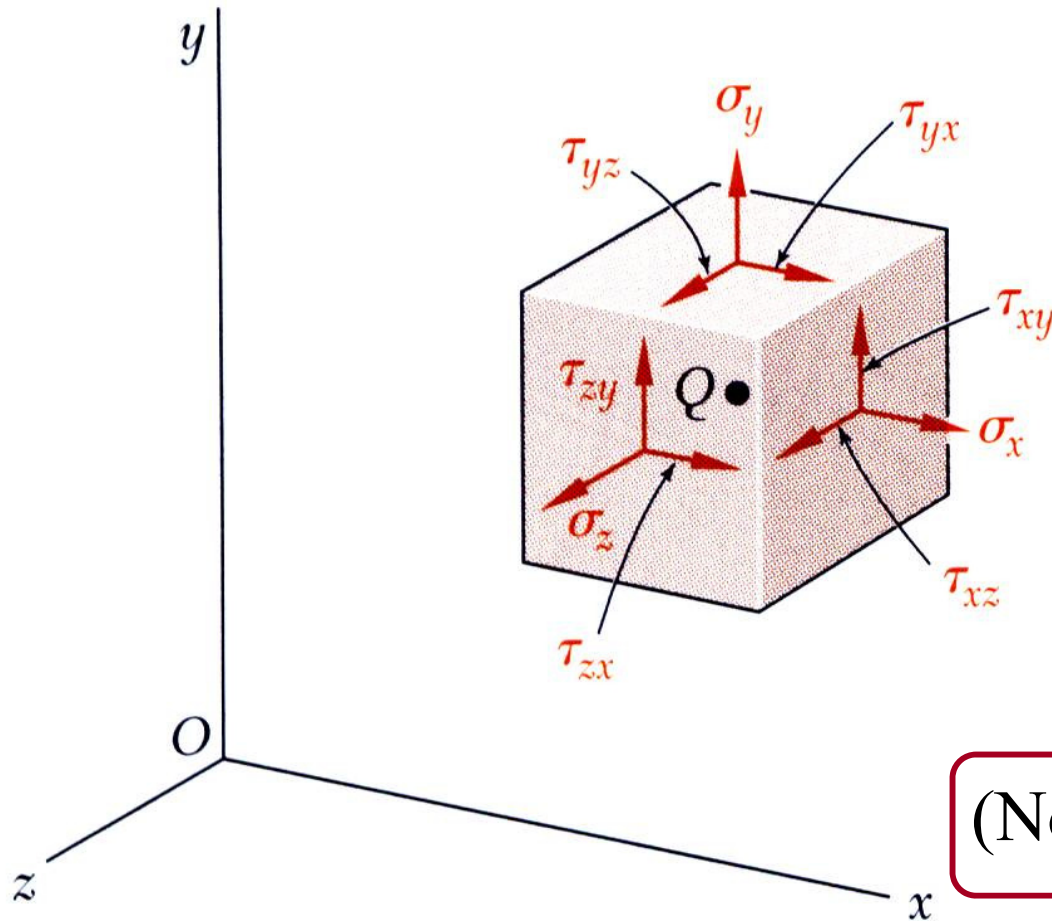
$$\sigma_z = \frac{dF_z}{dA}$$

$$\tau_{zx} = \frac{dF_x}{dA}$$

$$\tau_{zy} = \frac{dF_y}{dA}$$

Transformations of Stress and Strain

□ General or Triaxial State of stress



- Normal Stresses

$$\sigma_x, \sigma_y, \sigma_z$$

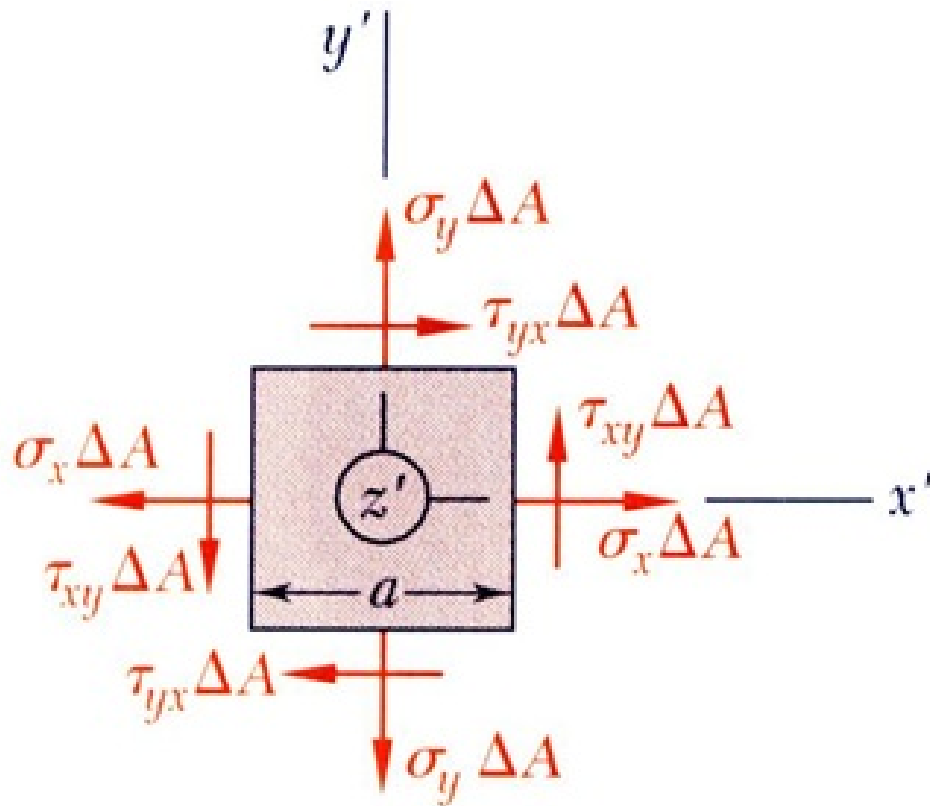
- Shear Stress

$$\tau_{xy}, \tau_{yz}, \tau_{zx}$$

(Note : $\tau_{xy} = \tau_{yx}$, $\tau_{yz} = \tau_{zy}$, $\tau_{zx} = \tau_{xz}$)

Introduction – Concept of Stress

□ Stress Under General Loadings



- Consider the moments about the z axis:

$$\sum M_z = 0 = (\tau_{xy} \Delta A) a - (\tau_{yx} \Delta A) a = 0$$

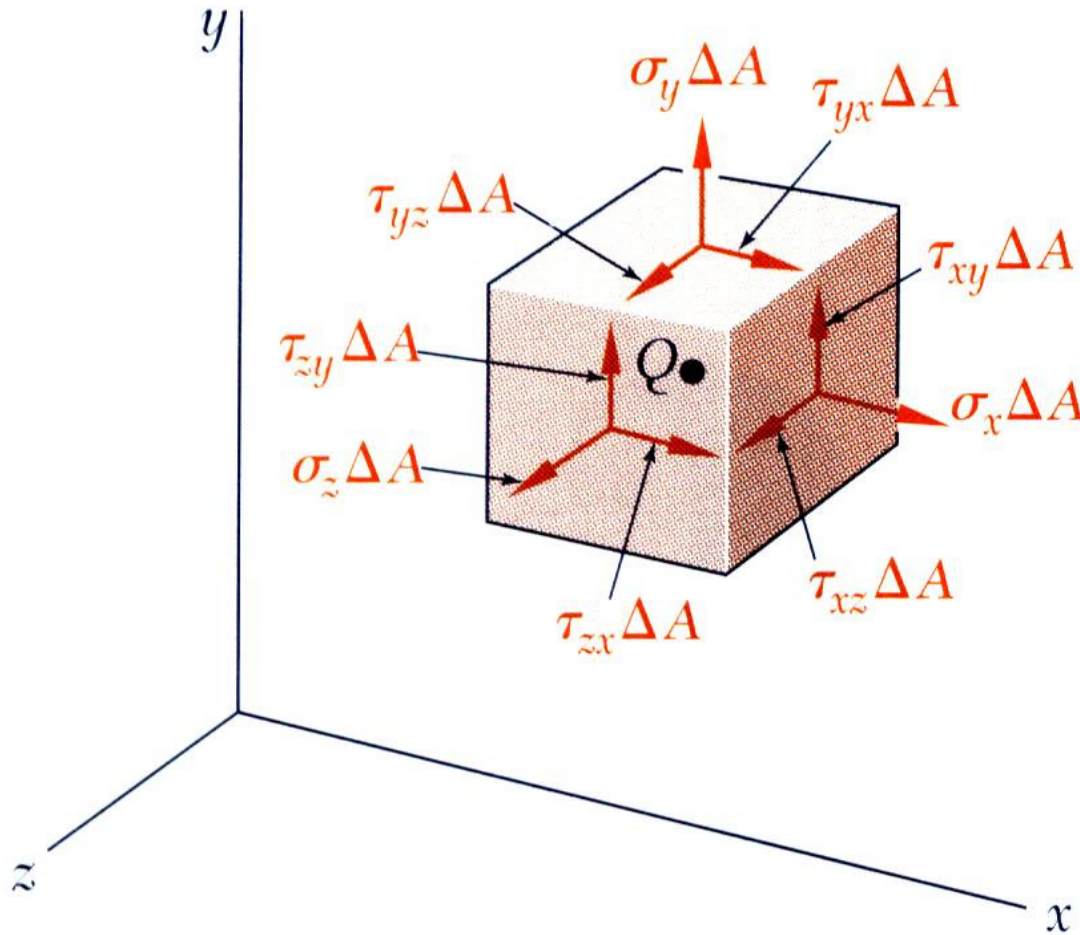
$$\Rightarrow \tau_{xy} = \tau_{yx}$$

similarly, $\tau_{yz} = \tau_{zy}$ and $\tau_{xz} = \tau_{zx}$

- It follows that only **6** components of stress are required to define the complete state of stress

Introduction – Concept of Stress

□ Stress Under General Loadings



- 9 stress components are unknown.

- Stress components are defined for the planes cut parallel to the x , y and z axes. For equilibrium, equal and opposite stresses are exerted on the hidden planes.
- The combination of forces generated by the stresses must satisfy the conditions for equilibrium:

$$\sum F_x = \sum F_y = \sum F_z = 0$$

$$\sum M_x = \sum M_y = \sum M_z = 0$$

Introduction –Concept of Stress

□ Factor of Safety

Structural members or machines must be designed such that the working stresses are less than the ultimate strength of the material.

FS = Factor of safety

$$FS = \frac{\sigma_u}{\sigma_{all}} = \frac{\text{ultimate stress}}{\text{allowable stress}}$$

Factor of safety considerations:

- uncertainty in material properties
- uncertainty of loadings
- uncertainty of analyses
- number of loading cycles
- types of failure
- maintenance requirements and deterioration effects
- importance of member to structures integrity
- risk to life and property
- influence on machine function

Introduction –Concept of Stress

□ Design Loads, Working Stresses, and Factor of Safety (FS)

Design Approaches:

- Deterministic, e.g., working stress or allowable stress design (ASD)

$$\frac{R_n}{FS} \geq \sum_{i=1}^m L_i$$

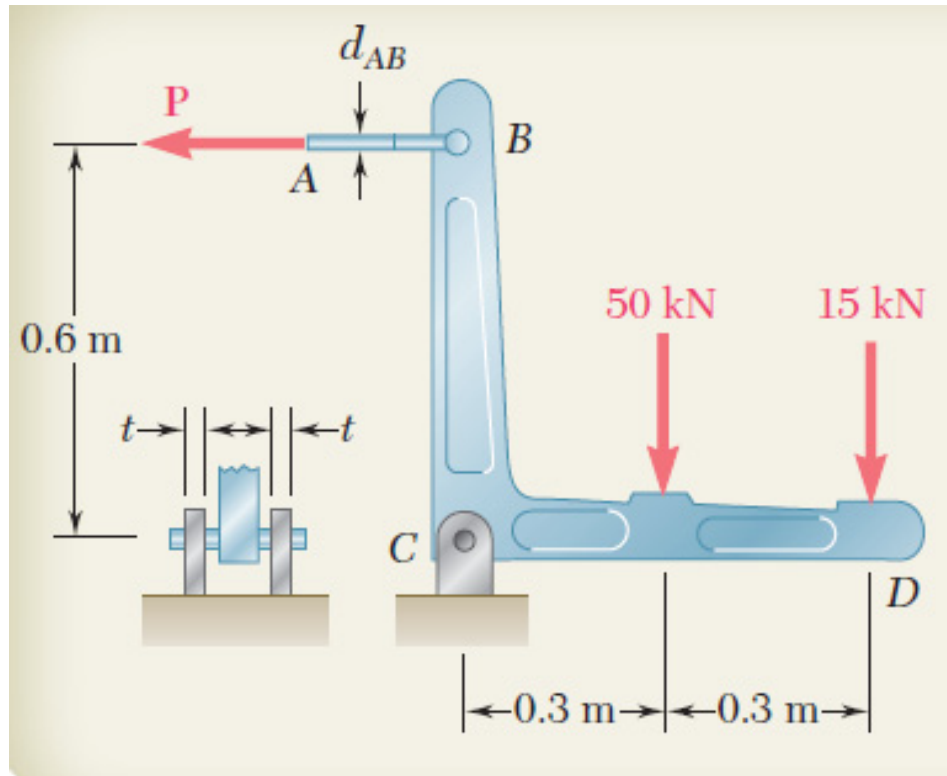
- Probability-Based, e.g., load and resistance factor design (LRFD)

$$\phi R_n \geq \sum_{i=1}^m \gamma_i L_i$$

Introduction – Concept of Stress

Example 6

□ Stress Analysis & Design Example

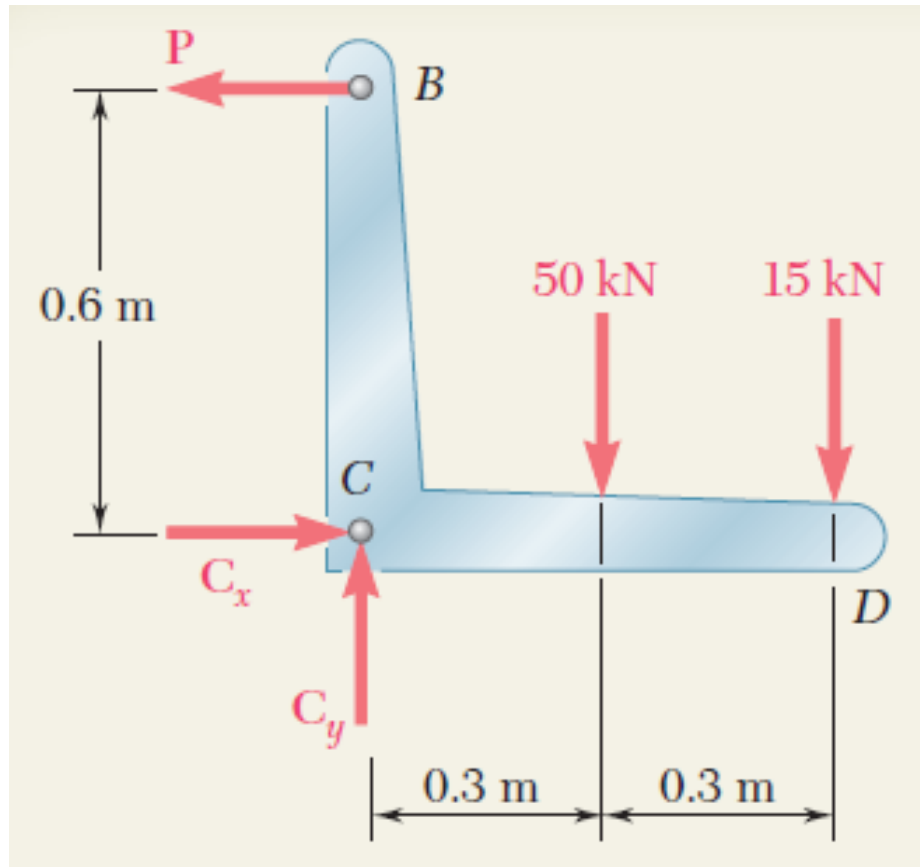


- a) Determine $d_{AB} = ?$
 $\sigma_U = 600 \text{ MPa}$ & $FS = 3.3$
- b) Determine $d_C = ?$
 $\tau_U = 350 \text{ MPa}$ & $FS = 3.3$
- c) Determine $t = ?$
 $\sigma_{all} = 300 \text{ MPa}$

Introduction – Concept of Stress

Example 6

□ Stress Analysis & Design Example

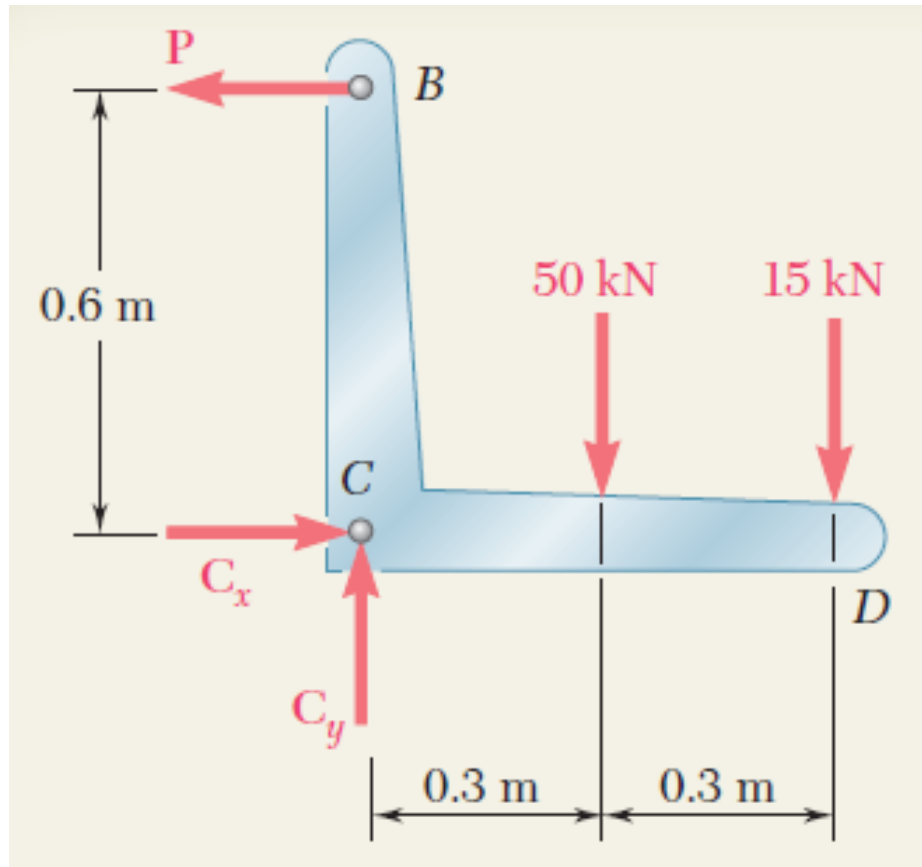


Introduction – Concept of Stress

Example 6

□ Stress Analysis & Design Example

a)

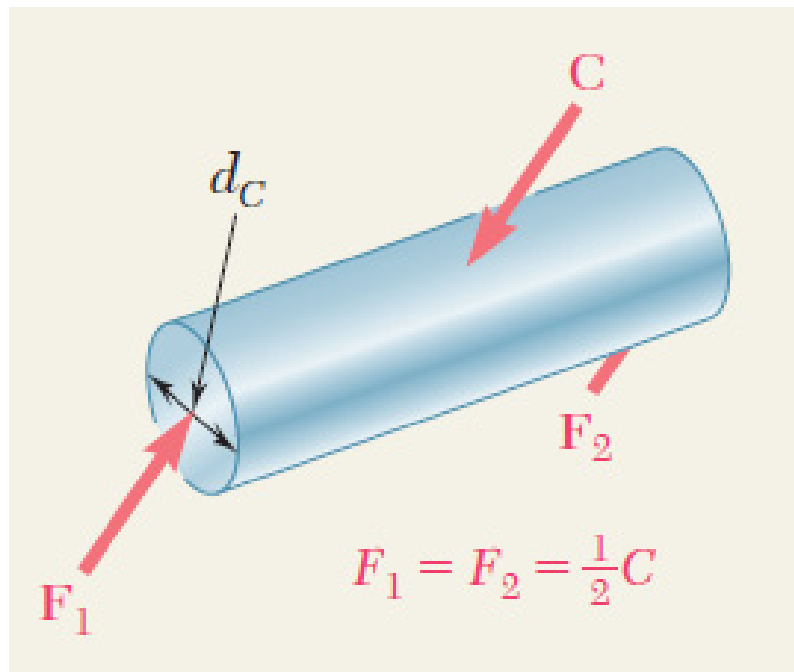


Introduction – Concept of Stress

Example 6

□ Stress Analysis & Design Example

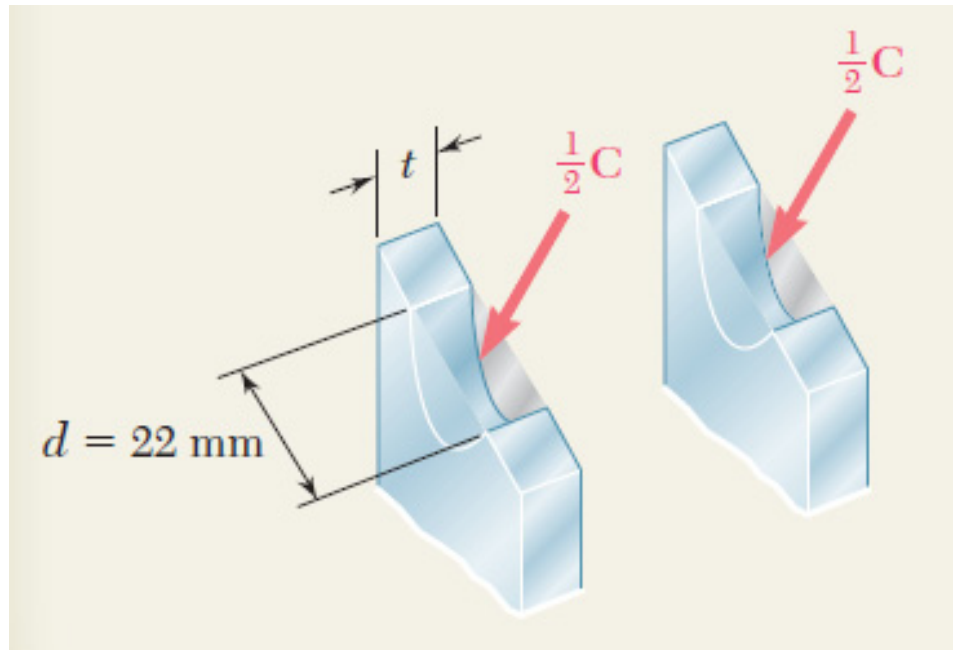
b)



Introduction – Concept of Stress

Example 6

□ Stress Analysis & Design Example



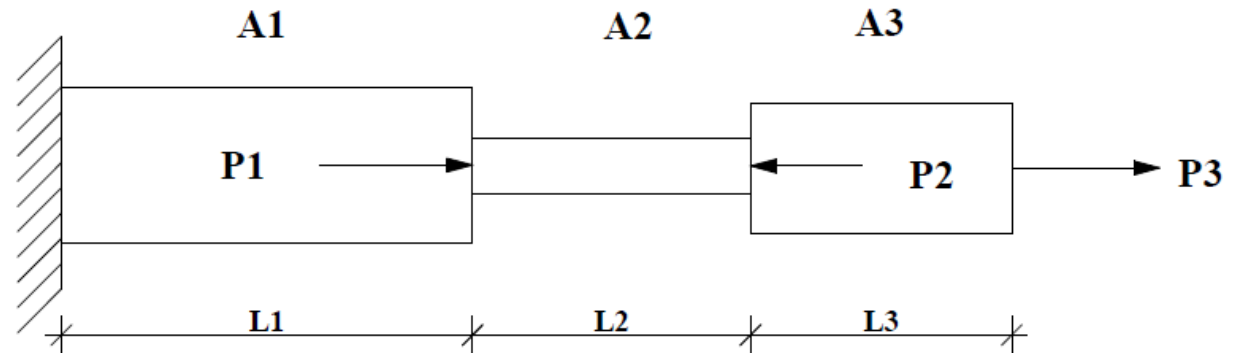
C)

Introduction –Concept of Stress

□ Stress Analysis & Design Example

Example 7

Determine axial forces and normal stresses in the shown bar. Also draw the variations of axial forces and normal stresses.



$$A_1 = 200 \text{ mm}^2 \quad P_1 = 4 \text{ kN}$$

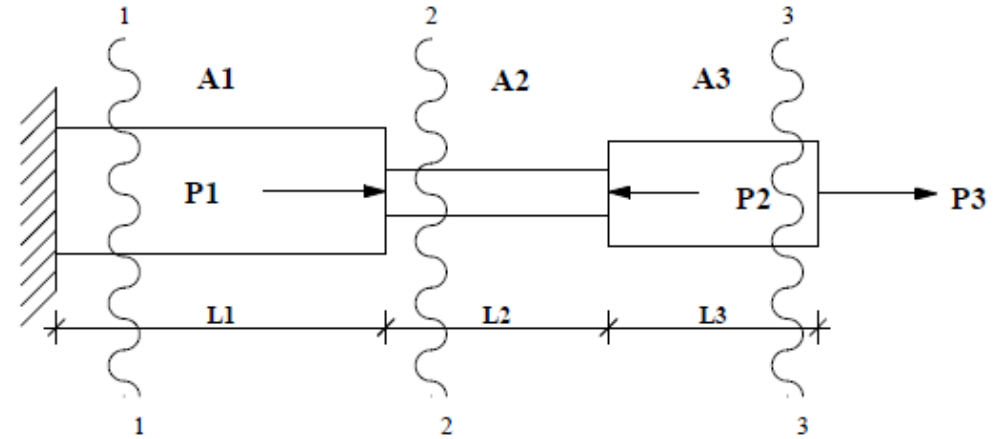
$$A_2 = 100 \text{ mm}^2 \quad P_2 = -2 \text{ kN}$$

$$A_3 = 150 \text{ mm}^2 \quad P_3 = 3 \text{ kN}$$

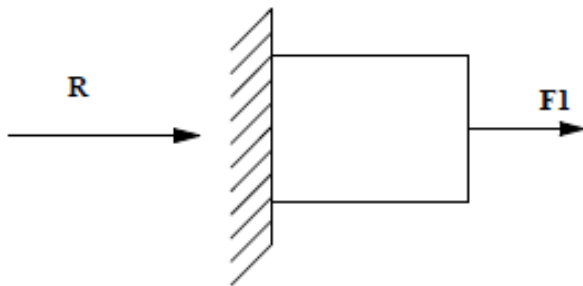
Introduction – Concept of Stress

□ Stress Analysis & Design Example

Example 7



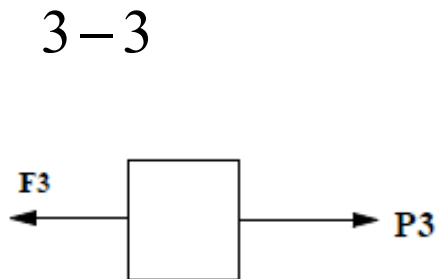
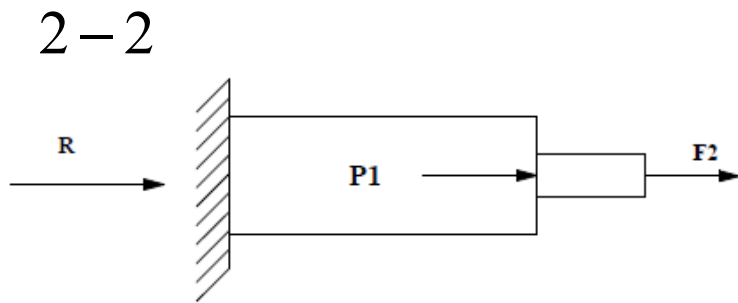
1-1



Introduction – Concept of Stress

□ Stress Analysis & Design Example

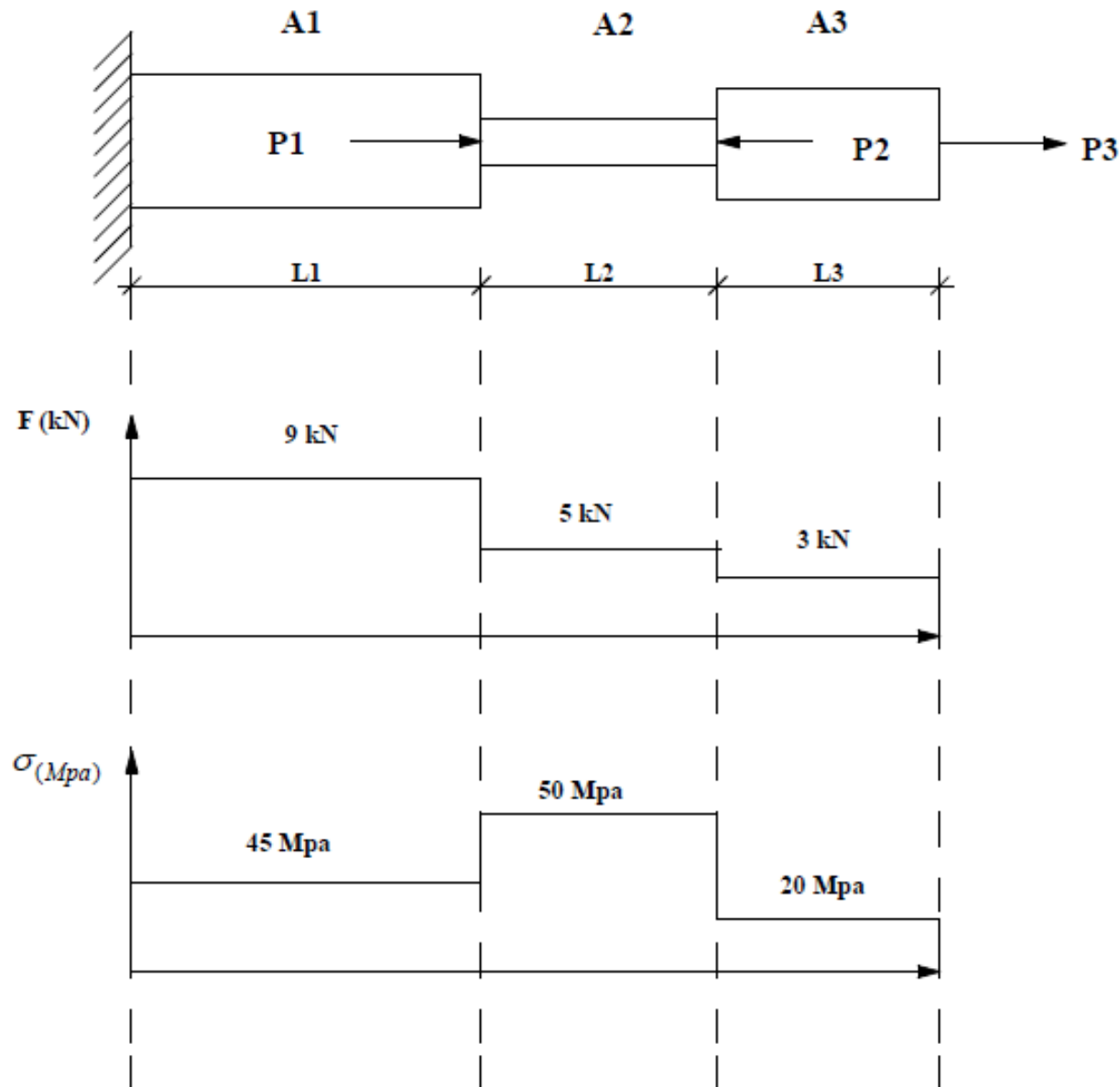
Example 7



Introduction – Concept of Stress

□ Stress Analysis & Design Example

Example 7

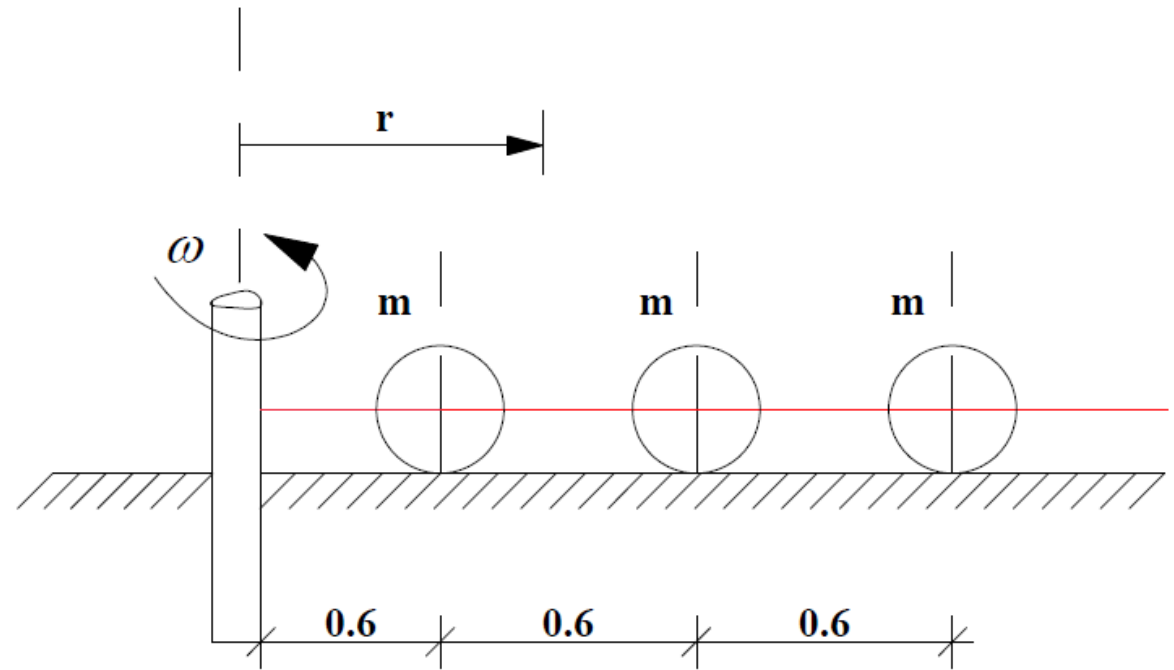


Introduction –Concept of Stress

□ Stress Analysis & Design Example

Example 8

Three lumped masses rotate around a beam. These masses are connected to each other using a wire with diameter d . determine axial stresses in parts of wire and draw the results based on r using a diagram. There is no friction between the surface and masses.

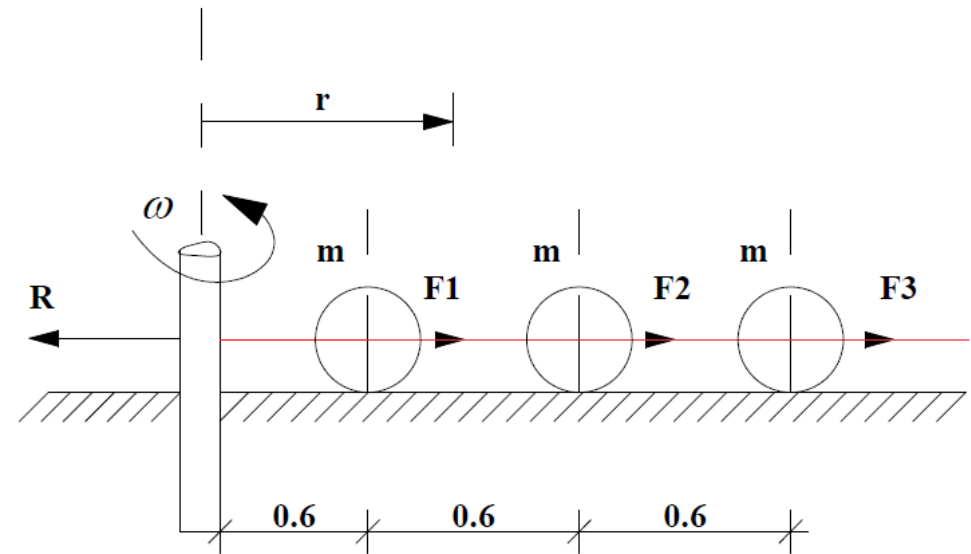


$$m = 0.5 \text{ kg} \quad \nu = 4 \text{ Hz} \quad d = 10 \text{ mm}$$

Introduction – Concept of Stress

□ Stress Analysis & Design Example

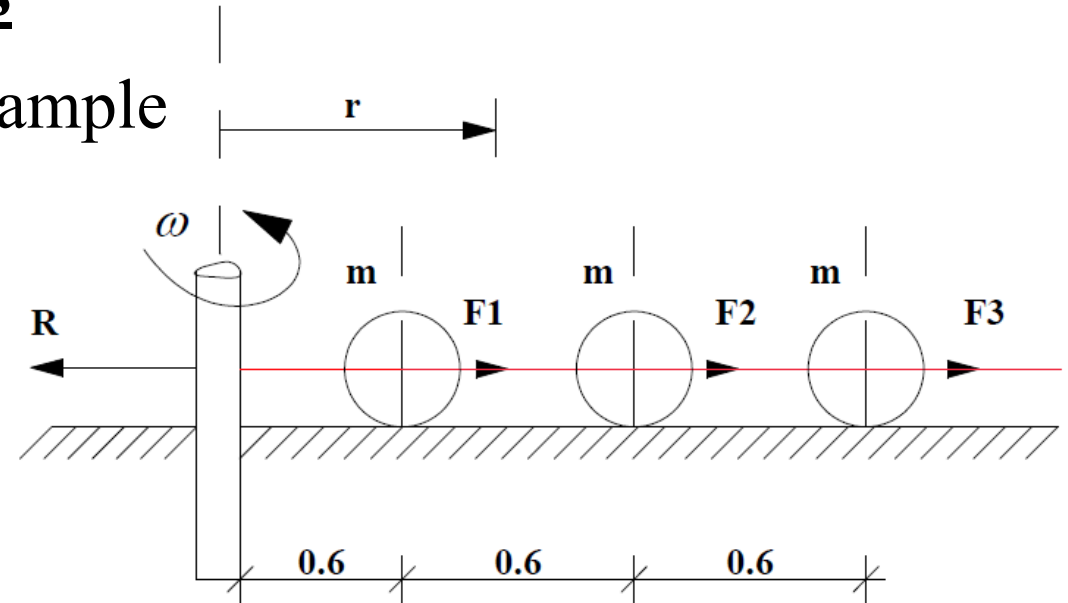
Example 8



Introduction – Concept of Stress

□ Stress Analysis & Design Example

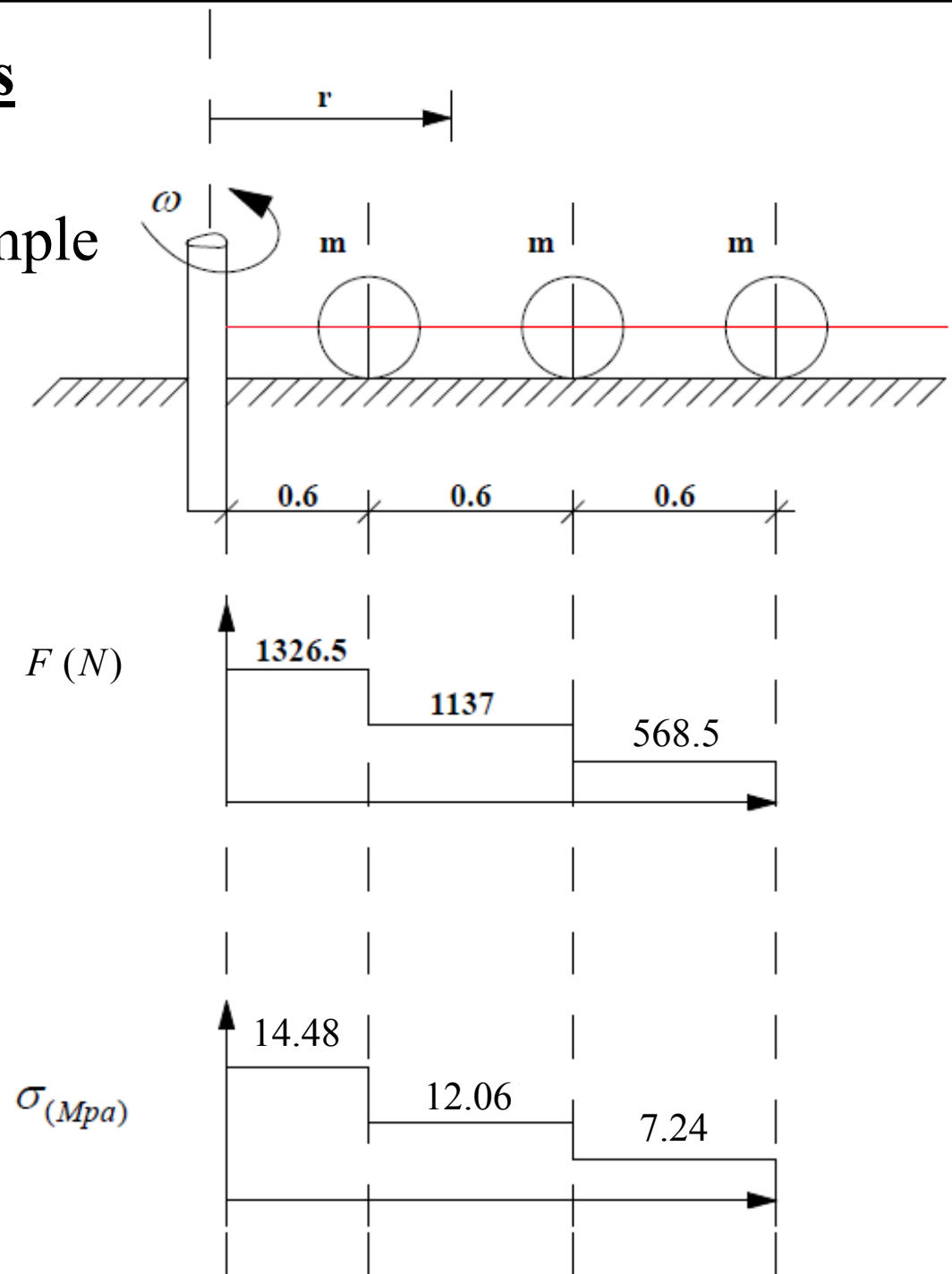
Example 8



Introduction – Concept of Stress

□ Stress Analysis & Design Example

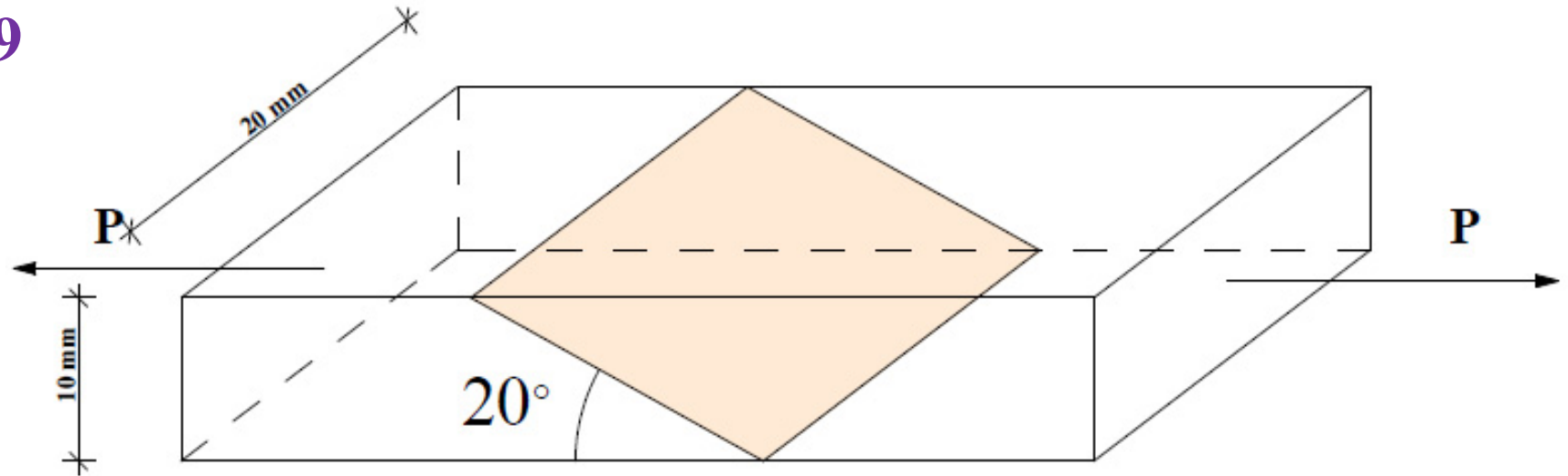
Example 8



Introduction –Concept of Stress

□ Stress Analysis & Design Example

Example 9



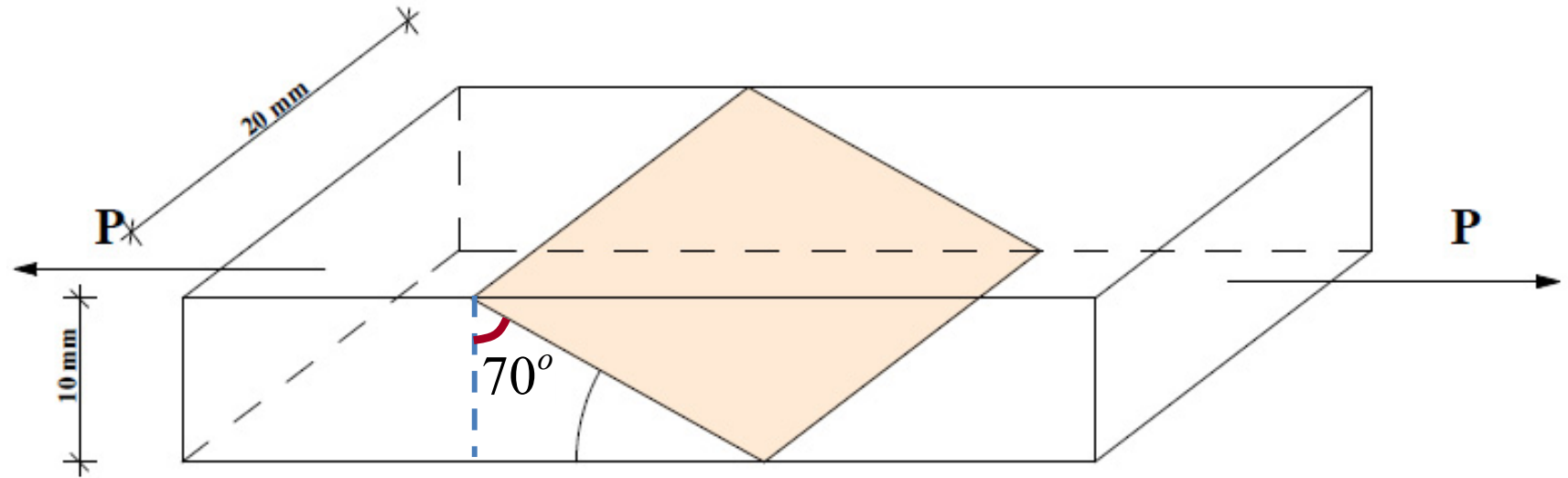
Two pieces of the wood beam are connected with glue connection. Determine the maximum safe axial load.

$$\tau_{all} = 10 \text{ Mpa}$$

Introduction – Concept of Stress

□ Stress Analysis & Design Example

Example 9



UNITS CONVERSION TABLES

Table 1: Multiples and Submultiples of SI units

Prefix	Symbol	Multiplying Factor	
exa	E	10^{18}	1 000 000 000 000 000 000
peta	P	10^{15}	1 000 000 000 000 000
tera	T	10^{12}	1 000 000 000 000
giga	G	10^9	1 000 000 000
mega	M	10^6	1 000 000
kilo	k	10^3	1 000
hecto*	h	10^2	100
deca*	da	10	10
deci*	d	10^{-1}	0.1
centi	c	10^{-2}	0.01
milli	m	10^{-3}	0.001
micro	u	10^{-6}	0.000 001
nano	n	10^{-9}	0.000 000 001
pico	p	10^{-12}	0.000 000 000 001
femto	f	10^{-15}	0.000 000 000 000 001
atto	a	10^{-18}	0.000 000 000 000 000 001

* these prefixes are not normally used

UNITS CONVERSION TABLES

Table 2: Length Units

Millimeters	Centimeters	Meters	Kilometers	Inches	Feet	Yards	Miles
mm	cm	m	km	in	ft	yd	mi
1	0.1	0.001	0.000001	0.03937	0.003281	0.001094	6.21e-07
10	1	0.01	0.00001	0.393701	0.032808	0.010936	0.000006
1000	100	1	0.001	39.37008	3.28084	1.093613	0.000621
1000000	100000	1000	1	39370.08	3280.84	1093.613	0.621371
25.4	2.54	0.0254	0.000025	1	0.083333	0.027778	0.000016
304.8	30.48	0.3048	0.000305	12	1	0.333333	0.000189
914.4	91.44	0.9144	0.000914	36	3	1	0.000568
1609344	160934.4	1609.344	1.609344	63360	5280	1760	1

Table 3: Area Units

Millimeter square	Centimeter square	Meter square	Inch square	Foot square	Yard square
mm ²	cm ²	m ²	in ²	ft ²	yd ²
1	0.01	0.000001	0.00155	0.000011	0.000001
100	1	0.0001	0.155	0.001076	0.00012
1000000	10000	1	1550.003	10.76391	1.19599
645.16	6.4516	0.000645	1	0.006944	0.000772
92903	929.0304	0.092903	144	1	0.111111
836127	8361.274	0.836127	1296	9	1

UNITS CONVERSION TABLES

Table 4: Volume Units

Centimeter cube	Meter cube	Liter	Inch cube	Foot cube	US gallons	Imperial gallons	US barrel (oil)
cm ³	m ³	ltr	in ³	ft ³	US gal	Imp. gal	US brl
1	0.000001	0.001	0.061024	0.000035	0.000264	0.00022	0.000006
1000000	1	1000	61024	35	264	220	6.29
1000	0.001	1	61	0.035	0.264201	0.22	0.00629
16.4	0.000016	0.016387	1	0.000579	0.004329	0.003605	0.000103
28317	0.028317	28.31685	1728	1	7.481333	6.229712	0.178127
3785	0.003785	3.79	231	0.13	1	0.832701	0.02381
4545	0.004545	4.55	277	0.16	1.20	1	0.028593
158970	0.15897	159	9701	6	42	35	1

Table 5: Mass Units

Grams	Kilograms	Metric tonnes	Short ton	Long ton	Pounds	Ounces
g	kg	tonne	shton	Lton	lb	oz
1	0.001	0.000001	0.000001	9.84e-07	0.002205	0.035273
1000	1	0.001	0.001102	0.000984	2.204586	35.27337
1000000	1000	1	1.102293	0.984252	2204.586	35273.37
907200	907.2	0.9072	1	0.892913	2000	32000
1016000	1016	1.016	1.119929	1	2239.859	35837.74
453.6	0.4536	0.000454	0.0005	0.000446	1	16
28	0.02835	0.000028	0.000031	0.000028	0.0625	1

UNITS CONVERSION TABLES

Table 10: High Pressure Units

Bar	Pound/square inch	Kilopascal	Megapascal	Kilogram force/centimeter square	Millimeter of mercury	Atmospheres
bar	psi	kPa	MPa	kgf/cm ²	mm Hg	atm
1	14.50326	100	0.1	1.01968	750.0188	0.987167
0.06895	1	6.895	0.006895	0.070307	51.71379	0.068065
0.01	0.1450	1	0.001	0.01020	7.5002	0.00987
10	145.03	1000	1	10.197	7500.2	9.8717
0.9807	14.22335	98.07	0.09807	1	735.5434	0.968115
0.001333	0.019337	0.13333	0.000133	0.00136	1	0.001316
1.013	14.69181	101.3	0.1013	1.032936	759.769	1

Table 16: Temperature Conversion Formulas

Degree Celsius (°C)	$(^{\circ}\text{F} - 32) \times 5/9$
	$(\text{K} - 273.15)$
Degree Fahrenheit (°F)	$(^{\circ}\text{C} \times 9/5) + 32$
	$(1.8 \times \text{K}) - 459.67$
Kelvin (K)	$(^{\circ}\text{C} + 273.15)$
	$(^{\circ}\text{F} + 459.67) \div 1.8$